

24m COMMUNICATION TOWER **BOWEN ORBITAL SPACEPORT**

TELCO ANTENNAS





DRAWING LIST				
SHEET NUMBER SHEET NAME				
S00	COVER PAGE			
S01	PROJECT NOTES			
S50	GENERAL ARRANGEMENT AND ELEVATION			
S100	FOOTING DETAILS			
S400	TOWER FRAMING PLAN AND ELEVATIONS			
S401	TOWER FRAMING DETAILS			

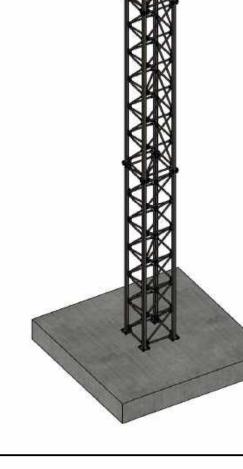
PROPOSED TOWER LOCATION SITE COORDINATES 19°57'30.98"S

PLANS AND DOCUMENTS referred to in the SDA APPROVAL

SDA approval: APC2022/007

PLANS AND DOCUMENTS referred to in the SDA APPROVAL

SDA approval: APC2024/006



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24m COMMUNICATION TOWER **BOWEN ORBITAL SPACEPORT**

TELCO ANTENNAS

COVER PAGE

Drawn		Date	Chkd	Date
	DJ		RL	
Design		Date	Apprd	Date
	MF		LS	
Scale		A3	Certif	Date
	NTS		LS	
Project	No.		Dwg. No.	Rev
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GENERAL NOTES

- THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL OTHER CONSULTANTS DRAWINGS AND SPECIFICATIONS
- BEFORE PROCEEDING WITH THE WORK ANY DISCREPANCIES IN THE CONTRACT DOCUMENTS SHALL BE REFERRED FOR DECISION TO THE ENGINEER.
- SETTING OUT DIMENSIONS AND SIZES OF STRUCTURAL MEMBERS SHALL NOT BE OBTAINED BY SCALING THE STRUCTURAL DRAWINGS
- ANY SETTING OUT DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE CHECKED BY THE CONTRACTOR BEFORE CONSTRUCTION COMMENCES.
- THE STRUCTURE HAS BEEN DESIGNED FOR THE FINAL INSERVICE CONDITION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND CERTIFICATION OF ALL TEMPORARY WORKS REQUIRED TO COMPLETE THE WORKS INCLUDING, RIGGING METHODOLOGY, TEMPORARY BRACING, PROPPING, SHORING, ASSESSMENT OF CONSTRUCTION LOADS AND ASSOCIATED WORK METHOD STATEMENTS, SUCH THAT THE CONSTRUCTION LOADS DO NOT EXCEED THE CAPACITY OF THE STRUCTURE. THE STRUCTURAL ENGINEER WILL NOT ASSESS, OR APPROVE ANY TEMPORARY WORKS, NOR ADVISE ON THE TIMING FOR THEIR
- ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT EDITIONS INCLUDING AMENDMENTS OF THE RELEVANT SAA STANDARDS, SAA CODES OF PRACTICE EXCEPT AS VARIED BY THE CONTRACT DOCUMENTS AND OF THE BY-LAWS OF THE LOCAL GOVERNMENT AUTHORITY

DESIGN CRITERIA

GENERAL PRINCIPLES:

- IMPORTANCE LEVEL

50 YEARS

TC2.0

Md = 0.9

Mc = 1.05

Ms = 1.0

Mt = 1.0

Mz,cat = 1.1

Vdes = 67.9m/s

azu = 2.77kPa

SELF-WEIGHT OF THE STEEL MEMEBERS AND CONNECTIONS HAVE BEEN ASSESSED IN ACCORDANCE WITH AS1170.1 PERMANENT ACTION DESIGN LOADS.

WIND LOADS HAVE BEEN ASSESSED IN ACCORDANCE WITH AS1170.2:

- REGION

- TERRAIN CATEGORY

- WIND DIRECTION MULTIPLIER

- CLIMATE CHANGE MULTIPLIER - TERRAIN/HEIGHT MULTIPLIER - SHIELDING MULTIPLIER

- TOPOGRAPHICAL MULTIPLIER - DESIGN WIND SPEED (ULS)

- DESIGN WIND PRESSURE (ULS)

- **FOUNDATION NOTES** FOOTINGS SHALL BE PLACED CENTRALLY UNDER COLUMNS UNLESS NOTED OTHERWISE
- EXCAVATIONS SHALL BE KEPT FREE OF PONDED WATER BEFORE PLACING CONCRETE.
- FOOTINGS HAVE BEEN DESIGNED BASED ON THE FOLLOWING GEOTECHNICAL REPORT:

GEOTECHNICAL DETAILS

GEOTECHNICAL REPORT BY	CQ Soil Testing
REPORT JOB NUMBER	CQ20875
REPORT DATE	6/04/2022
SITE CLASSIFICATION TO AS2870	H2
CHARACTERISTIC SURFACE MOVEMENT (Ys)	60-70 mm
MIN. BEARING CAPACITY (ULS)	350kPa
EXPECTED FOUNDING MATERIAL	VERY STIFF SILTY CLAY

- GEOTECHNICAL ENGINEER TO CONFIRM FOUNDATION DESIGN PARAMETERS PRIOR TO PLACING
- PLACE BLINDING CONCRETE TO BASE OF FOOTING EXCAVATION TO ACHIEVE DESIGN AND / OR UNIFORM BEARING MATERIAL

EARTHWORKS NOTES

- REMOVE ALL TOPSOIL FROM THE AREA OF THE WORKS. TOPSOIL TO BE STOCKPILED AND REUSED AS REQUIRED OR REMOVED FROM SITE.
- REMOVE BOULDERS AND ROCKS WITHIN 100mm OF UNDERSIDE OF OCNCRETE SLABS AND PAVEMENTS.
- CUT SURFACE TO BE COMPACTED TO 98% STANDARD COMPACTION.
- ANY SETTING OUT DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE CHECKED BY THE CONTRACTOR BEFORE CONSTRUCTION COMMENCES.

GROUT NOTES

- DUE TO HIGHLY REACTIVE SOIL FOUNDATION THE TOWER MAY NEED RESETTING VERTICALLY FROM TIME TO TIME BY REMOVING THE GROUT, ADJUSTING THE ANCHOR BOLT NUTS AND
- GROUT IS TO BE CEMENTITIOUS. HIGH STRENGTH AND NON-SHRINK (FOSROC CONBEXTRA BB92
- INSTALLATION IS TO BE IN ACCORDANCE WITH MANUFACTURES SPECIFICATION

PLANS AND DOCUMENTS referred to in the SDA APPROVAL



SDA approval: APC2022/007

CONCRETE NOTES

ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS 3600 & AS 1379.

CONCRETE QUALITY

ELEMENT MAX SIZE AGGREGATE

CONCRETE GRADE

CLEAR CONCRETE COVER TO REINFORCEMENT (OUTSIDE OF STIRRUPS AND TIES) SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE

> **CLEAR COVER FOOTING**

SPECIFIED STRUCTURAL THICKNESSES FOR CONCRETE ELEMENTS DO NOT INCLUDE THICKNESS OF APPLIED FINISHES.

FOOTING

- CONSTRUCTION JOINTS OR POUR BREAKS WHERE NOT SHOWN ON PLANS OR DETAILS SHALL BE LOCATED AND FORMED TO THE APPROVAL OF THE ENGINEER
- NO PENETRATIONS, RECESSES OR CHASES OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE MADE IN CONCRETE MEMBERS WITHOUT APPROVAL OF THE ENGINEER.
- AT PENETRATIONS IN SLABS UNLESS OTHERWISE DETAILED REINFORCEMENT MUST NOT BE CUT BUT SHALL BE GATHERED EQUALLY TO EACH SIDE OF PENETRATION AND EXTRA REINFORCEMENT PROVIDED BETWEEN THE PENETRATIONS AS DIRECTED BY THE ENGINEER.
- REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY. IT IS NOT NECESSARILY SHOWN
- PROVIDE THE ENGINEER WITH 48 HOURS NOTICE OF REINFORCEMENT BEING READY FOR INSPECTION. NO CONCRETE IS TO BE POURED WITHOUT THE APPROVAL OF THE ENGINEER. THIS IS TO BE CONFIRMED AT A LATER DATE.
- SPLICES IN REINFORCEMENT SHALL BE MADE ONLY IN THE POSITIONS SHOWN, WHERE LAP LENGTH IS NOT SHOWN IT SHALL BE SUFFICIENT TO DEVELOP THE FULL STRENGTH OF THE REINFORCEMENT. THE FOLLOWING MINIMUM SPLICE LENGTHS SHALL BE USED

THE FOLLOWING MINIMUM BAR SPLICE LENGTHS ARE APPLICABLE FOR fc >= 25MPa. A - FOR VERTICAL OR HORIZONTAL BARS WITH LESS THAN 300mm OF CONCRETE CAST

BAR DIAMETER LAP LENGTH	N10 360mm	N12 440mm	N16 605mm	N20 865mm	N24 1165mm	N28 1485mm
BAR DIAMETER	N32	N32				
LAP LENGTH	1835mm	2210mm				
B - FOR HORIZONTAL B BELOW	ARS GREAT	ER THAN OI	R EQUAL TO	300mm OF C	CONCRETE C	AST
BAR DIAMETER	N10	N12	N16	N20	N24	N28
LAP LENGTH	470mm	570mm	785mm	1125mm	1550mm	1958mm
BAR DIAMETER	N32	N36				

- 11. WELDING OF REINFORCEMENT WILL ONLY BE PERMITTED WITH THE PRIOR APPROVAL OF
- REINFORCEMENT MUST NOT BE CONTINUOUS THROUGH CONTRACTION JOINTS.

2385mm 2870mm

- PLACE SUFFICIENT STOOLS LINDER MAIN BOTTOM REINFORCING RODS AND TOP CROSS RODS IN SLABS TO ALLOW THEM TO BE SUPPORTED IN THEIR CORRECT POSITIONS DURING CONCRETING (NOT GREATER THAN 900mm).
- 14. REINFORCEMENT SYMBOLS

LAP LENGTH

SPECIFICATION SYMBOL GRADE D500N DEFORMED BAR STRUCTURAL GRADE ROUND BAR RF AND SL HARD DRAWN SHEET RIBBED WIRE REINFORCING FABRIC

- THE NUMBER FOLLOWING THESE SYMBOLS IS THE BAR DIAMETER IN MILLIMETERS. SLABS TO BE CURED BY COVERING WITH 0.20mm BLACK POLYETHYLENE SHEETING AND KEPT MOIST FOR 7 DAYS MINIMUM.
- NO CONCRETE TO BE POURED WHEN SITE TEMPERATURE EXCEEDS 35°C OR FALLS
- SUPPLY AND LAY FABRIC IN FLAT SHEETS AT SPLICES. FABRIC SHALL BE LAPPED AS

COATING NOTES

- THE FOLLOWING COATING REQUIREMENTS HAVE BEEN DETERMINED BY AS4312.
- ALL FABRICATED STRUCTURAL STEELWORK TO BE PREPARED WITH ABRASIVE BLAST
- SURFACES REQUIRING ONSITE TREATMENT ARE TO BE PRIMED IN ACCORDANCE WITH SURFACE TREATMENT MANUFACTURERS REQUIREMENTS
- COATINGS HAVE BEEN SPECIFIED BASED ON THE FOLLOWING

CORROSIVITY CATERGORY	C3 (MEDIUM)
STEEL CORROSION RATE µm/y	25-50
TYPICAL ENVIRONMENT	COASTAL OR INDUSTRIAL
DURABILITY CLASS	EL (EXTRA LONG TERM)
LIFE TO FIDET MAINTENANCE	EO VEADO

PLANS AND DOCUMENTS referred to in the SDA APPROVAL

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STRUCTURAL STEELWORK NOTES

- ALL STEELWORK TO BE IN ACCORDANCE WITH AS4100 UNO
- ALL WELDING TO BE IN ACCORDANCE WITH AS 1554 EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS. WELDING CONSUMABLES TO HAVE A MINIMUM NOMINAL TENSILE STRENGTH fuw = 490MPa. ALL WELDS NOT DESIGNATED ON PLAN SHALL BE 6mm GENERAL PURPOSE FILLET WELDS. COMPLETE PENETRATION BUTT WELDS (CPBW) SHALL BE STRUCTURAL PURPOSE WELDS
- 3. UNLESS NOTED OTHERWISE, ALL STEEL SHALL BE IN ACCORDANCE WITH:

BHP GRADE 300 PLUS FOR UNIVERSAL BEAMS AND COLUMNS FOR ALL PLATES AND CLEATS

FOR SHS AND RHS SECTIONS GR350 OR 450 AS APPLICABLE

AS1163 FOR CHS SECTIONS GR250 OR 350 AS APPLICABLE

- 4. THE CONTRACTOR SHALL PREPARE FABRICATION DRAWINGS AND SUBMIT TO THE ENGINEER FOR REVIEW. FABRICATION IS NOT TO COMMENCE UNTIL FABRICATION DRAWINGS HAVE BEEN APPROVED
- ALL STEELWORK THAT WILL BE EXPOSED TO VIEW IS TO HAVE WELD SPATTER, FLUX, DAGS AND BURRS REMOVED, AND ALL SEALING AND BUTT WELDS GROUND FLUSH.
- THE BUILDER SHALL PROVIDE ALL CLEATS AND HOLES FOR FIXING ALL BUILDING ELEMENTS TO STEEL AS REQUIRED BY THE DRAWINGS WHETHER OR NOT SHOWN.
- 4.6/S COMMERCIAL BOLT GRADE 4.6 TO AS1111 SNUG TIGHT. 8.8/S HIGH STRENGTH STRUCTURAL BOLT GRADE 8.8 TO AS1252 SNUG
- 8.8/TB HIGH STRENGTH STRUCTURAL BOLT GRADE 8.8 TO AS1252 TENSIONED TO AS4100 BEARING JOINT.
- 8.8/TF HIGH STRENGTH STRUCTURAL BOLT GRADE 8.8 TO AS1252 TENSIONED TO AS4100 FRICTION JOINT.
- ALL BOLTS TO BE HOT DIP GALVANISED EXCLUDING PURLIN BOLTS.
- ALL BOLTED CONNECTIONS OF FRICTION GRIP TYPE SHALL HAVE CONNECTION
- MECHANICAL PROPERTIES AND PROCEDURES CERTIFICATES FOR ALL MATERIALS, BOLTS AND WELDS SHALL BE MAINTAINED BY THE MAIN CONTRACTOR AND PROVIDED ON
- 11. REFER TO ARCHITECTURAL DETAILS FOR ADDITIONAL NON-STRUCTURAL STEELWORK.

ERECTION NOTES

- THE CONTRACTOR / INSTALLER IS TO ENSURE THE TOWER IS MAINTAINED IN A STABLE CONDITION AND THAT NO PART OF THE STRUCTURE IS OVERSTRESSED DURING
- INSTALLATION OF TOWER TO BE ERECTED IN ACCORDANCE WITH AS4100.
- ENSURE ALL BOLT HEADS & NUTS SIT FLAT ON FLANGES.
- ALL FLANGES MUST MAKE FULL CONTACT AT EACH BOLT PRIOR TO 'SNUG-TIGHT' BOLTS. ADVISE DESIGNER IF THIS IS NOT ACHIEVED.
- ERECTION TOLERANCES SHALL BE LIMITED TO: VERTICALITY - LESS THAN H/360

TWIST - LESS THAN 2° PER MODULE

TOWER FARTHING DESIGNED & SPECIFIED BY OTHERS.

TOWER ACCESS

STRUCTURAL ELEMENTS HAVE BEEN ASSESSED FOR LOADS GENERATED FROM CLIMBING/WORKING AT HEIGHTS. A SINGLE PERSON LOAD OF 15kN IS APPLIED IN ACCORDANCE WITH TABLE 3.1 IN

AS1891.4 REQUIRES THAT SIGNAGE BE INSTALLED AT THE TOWER BASE DESCRIBING WHICH AREAS OF THE STRUCTURE CAN BE USED FOR ANCHORAGE.

- 1. THE TOWER SHALL ONLY BE ASCENDED BY QUALIFIED AND COMPETENT PERSONNEL
- FALL ARREST DEVICES MUST ONLY BE ATTACHED TO THE LEG MEMBER OR A DIAGONAL CROSS BRACES OR FALL ARREST ATTACHMENT POINT NOMINATED ON THE DRAWINGS.
- PERSONNEL CLIMBING THE TOWER ARE TO HAVE APPROPRIATE CLIMBING GEAR TO SUIT THE LEG OR BRACING MEMBERS.
- ONLY THE BRACE MEMBERS SHOWN IN Purple IN THE GENERIC DIAGRAM BELOW CAN BE USED AS A CLIMBING/WORK AT HEIGHTS ANCHOR POINT.
- THE SPECIFIED ANCHOR POINTS ARE LIMITED TO ONE (1) PERSON AT ANY TIME.
- PRIOR TO ANCHORING TO ANY ELEMENT, VISUALLY CHECK CONNECTIONS ARE COMPLETE AND HAVE NO DAMAGE. ANY DEFECTS SHOULD BE REPORTED TO THE MANUFACTURER
- ANY STRUCTURAL MEMBER MAY BE USED TO PHYSICALLY MOVE AROUND THE STRUCTURE
- DO NOT ATTACH FALL ARREST DEVICES TO HORIZONTAL CROSS BRACING
- DO NOT ATTACH FALL ARREST DEVICES TO BRACING OR ATTACHMENT POINTS THAT EXHIBITS DAMAGE, RUST OR CRACKING



FIGURE 1

FIGURE 1 - LANYARD HOOK SAFE PLACEMENT (PURPLE SHADING)

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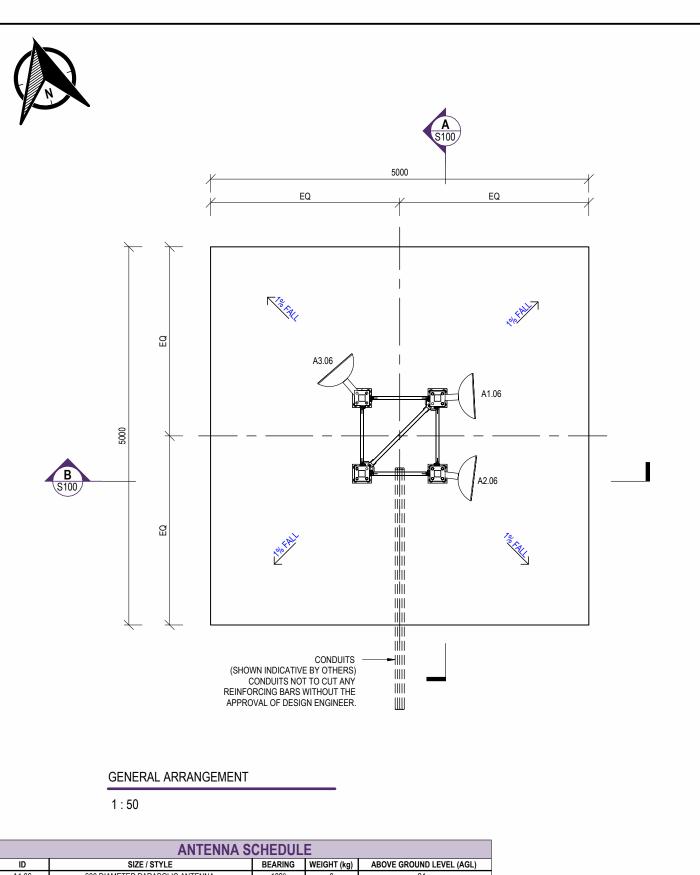
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TELCO ANTENNAS

PROJECT NOTES

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DJ		RL	
Design	Date	Apprd	Date
MF		LS	
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NTS		LS	
Project No.		Dwg. No.	Rev
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	ANTENNA S	CHEDUL	E	
ID	SIZE / STYLE	BEARING	WEIGHT (kg)	ABOVE GROUND LEVEL (AGL)
A1.06	600 DIAMETER PARABOLIC ANTENNA	102°	8	24m
A2.06	600 DIAMETER PARABOLIC ANTENNA	112°	8	24m
A3.06	600 DIAMETER PARABOLIC ANTENNA	334°	8	24m
FUTURE	600 DIAMETER PARABOLIC ANTENNA	(TBC)	8	22m
FUTURE	600 DIAMETER PARABOLIC ANTENNA	(TBC)	8	22m

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BOTTOM OF BASE PLATE
0.00

ELEVATION 1:100



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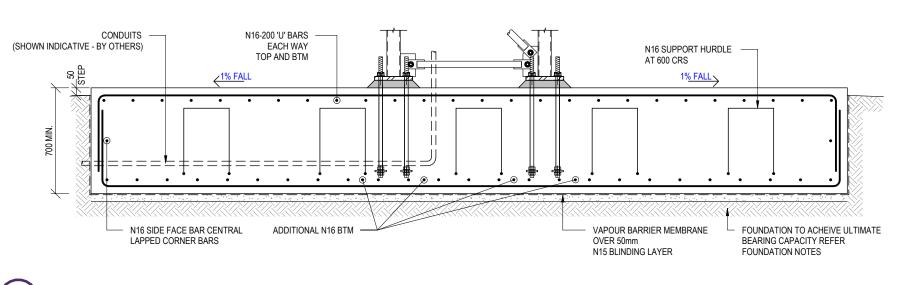
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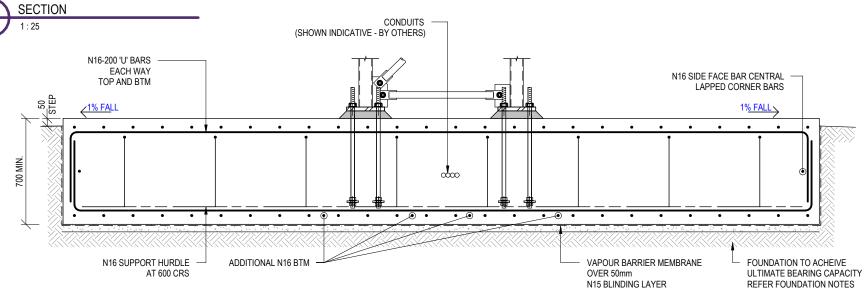
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GENERAL ARRANGEMENT AND **ELEVATION**

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DJ		RL	
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N16 SUPPORT HURDLE

SUPPORT HURDLE DETAIL

1:25

B SECTION \$50 1:25

HD BOLT DETAIL - THREADED ROD

BOLT SIZE (D)

		HOLD DOWN BOL	T	
	BOLT		WASHE	₹
BOLT DIAMETER (D)	EMBEDMENT (E)	PROJECTION (P)	SQUARE WASHER (W)	THICKNESS (T)
30	550	200	100	25

PLANS AND DOCUMENTS referred to in the SDA APPROVAL

LEAVE PROJECTION FOR FUTURE ADJUSTMENT. PROVIDE PLASTIC SAFETY CAPS

GRADE 4.6/S THREADED ROD, NUTS AND

BASE PLATE AND GROUT

TOP OF CONCRETE



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PLANS AND DOCUMENTS referred to in the SDA APPROVAL

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24m COMMUNICATION TOWER BOWEN ORBITAL SPACEPORT

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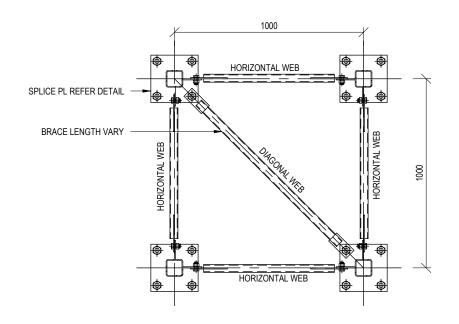
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FOOTING DETAILS

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DJ		RL	
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NTS		LS	
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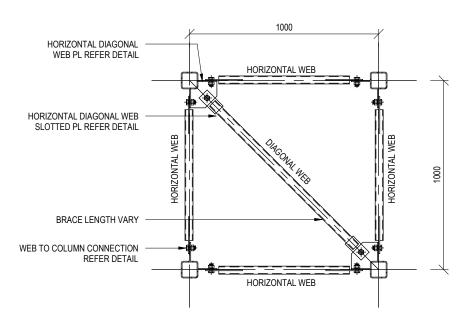
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1:10



TYPICAL SECTION AT SPLICE

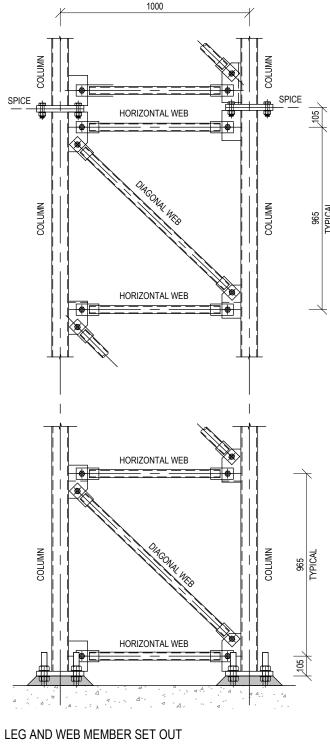
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TYPICAL SECTION AT MID-SECTION

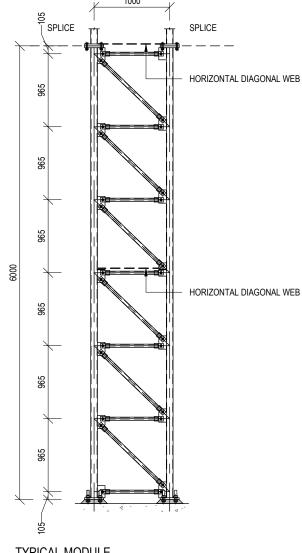
1:20

MEMBER SCHEDULE					
MODULE	MEMBER	DESCRIPTION	GRADE		
MODULE A	COLUMN	89x89x6.0 SHS	350		
MODULE B TO D	COLUMN	89x89x3.5 SHS	350		
MODULE A TO D	HORIZONTAL AND DIAGONAL WEB	42.4x3.2 CHS	250		



1:20

ALL LEG TO SPLICE/BASE PLATE CONNECTIONS TO BE COMPLETE PENETRATION BUTT WELDS. ALL OTHER WELDS TO REMAIN 6mm CFW (U.N.O.).



TYPICAL MODULE

1:50

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TOWER FRAMING PLAN AND **ELEVATIONS**

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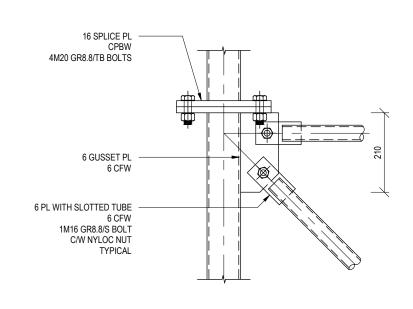
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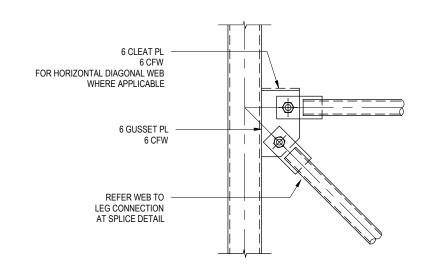
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WEB TO LEG CONNECTION AT SPLICE

1:10



DIAGONAL WEB TO SPLICE PLAN DETAIL

REFER DETAIL

1:10

REFER WEB TO

LEG CONNECTION AT SPLICE DETAIL

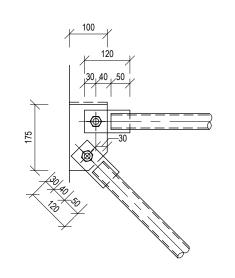
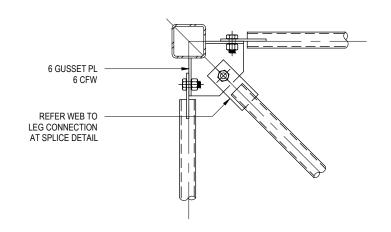


PLATE SET OUT

1:10

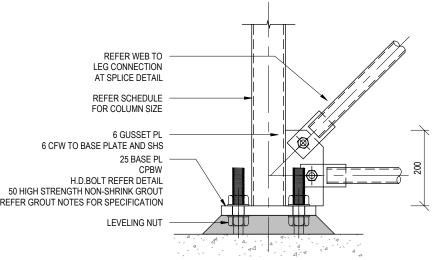


DIAGONAL WEB TO LEG PLAN DETAIL

1:10

WEB TO LEG CONNECTION

1:10



BASE PLATE DETAIL

1:10



REFER MEMBER SCHEDULE FOR COLUMN SIZE BASE PLATE REFER DETAIL

H.D. BOLT REFER H.D BOLT DETAIL

TYPICAL BASE PLATE

1:10

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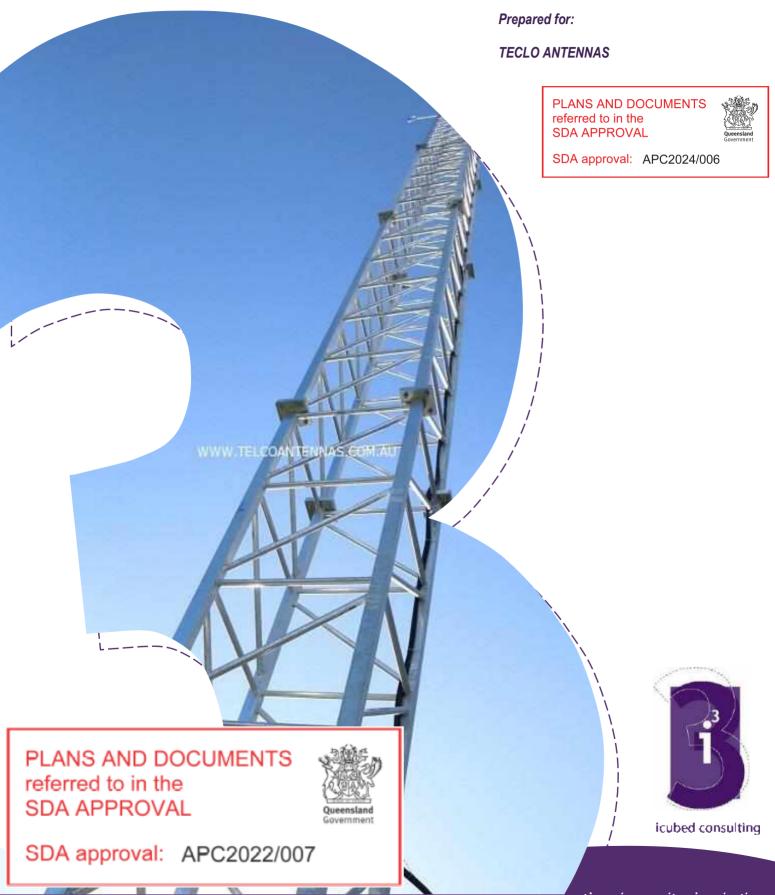
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STRUCTURAL COMPUTATION REPORT

22-055 –24m COMUNICATION TOWER AT BOWEN ORBITAL SPACEPORT



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Client / Project				Job No.	
Telco Antennas / 24m Steel Tower at Bowen Orbital Spaceport			22-055		
Section				Sheet No./Rev.	
Structural Computation Report				2/1	
Calc. by	Date	Chk'd by	Date	App'd by	Date
MF	04/07/22	LS	04/07/22	LS	04/07/22

Document1	
Prepared By	MF
Approved By	LS
Date	04/07/22
Job Number	22-055
Document Name	Structural Computation Report
Version	V 1.0

Document Revision History

Version	Revision History		
1.0	Initial issue		

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	Section				Sheet No./Rev.	
	Structural Computation Report				3 / 1	
ĺ	Calc. by	Date	Chk'd by	Date	App'd by	Date
	MF	04/07/22	LS	04/07/22	LS	04/07/22

	TABLE OF CONTENTS		
1	Scope	4	
	Design scope:	4	
2	Client Specification	5	
2	2.1 Site Location		
3	Applicable Design Standards	6	
4	Design Actions	7	
	I.1 General Principals		
	I.2 Permanent Actions		
	I.4 Wind Actions		
5	Tower Climbing Adequacy	9	
6	Safety In Design	10	
7	Structural Analysis & Results	11	
7	7.1 Lateral Displacement		
-	7.2 Tower Base Reactions		
	7.3 Geotechnical Parameters		
/	7.4 Footing Design		



Client / Project				Job No.	
Telco Antennas / 24m Steel Tower at Bowen Orbital Spaceport			22-055		
Section				Sheet No./Rev.	
Structural Computation Report				4/1	
Calc. by	Date	Chk'd by	Date	App'd by	Date
MF	04/07/22	LS	04/07/22	LS	04/07/22

1 Scope

I3 consulting was engaged by Telco Antennas to undertake a structural design and certification of the 1m face steel tower and footing for the proposed project at Orbital Spaceport Bowen QLD. This 24m high tower is to support communication equipment (ie. parabolic antennas)

1.1 Design scope:

The following structural design and certification was performed on the steel tower structure, connections and concrete footing:

- Calculate wind loads for the proposed orbital spaceport site in accordance with AS1170.2-2021
- Determine wind loads on the tower and the specific set of antennas.
- Structural model and analysis of the proposed tower
- Design check steel member sizes and connections for the bolted web members
- Design reinforced concrete pad footing
- Structural drawings of the footings, steel towers and connection details
- Design computation report and design compliance statement (form 15)

1.2 Structural Assessment & Certification Limitations

This design and certification is based on the fabrication and installation of the steel tower being carried out in accordance with i3 computation report and drawings 22-055 – S00, S01, S50, S100, S400 and S401.



Client / Project	•				
Telco Antennas / 24m Steel Tower at Bowen Orbital Spaceport			22-055		
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Section				Sheet No./Rev.	
Structural Computation Report					5/1
Calc. by	Date	Chk'd by	Date	App'd by	Date
MF	04/07/22	LS	04/07/22	LS	04/07/22

2 Client Specification

Client provided information for the proposed 24m steel framed tower

- Geometry of 1m square steel frame tower
- Proposed antennas size, above ground level and bearings refer antenna schedule in section 4.2

2.1 Site Location

The proposed steel tower is to be installed at Orbital Spaceport Bowen, Queensland with the following GPS co-ordinates:





Ī	Client / Project				Job No.	
	Telco Antennas / 24m Steel Tower at Bowen Orbital Spaceport			22-055		
ł	Section				Sheet No./Rev.	
	Structural Computation Report				6 / 1	
İ	Calc. by	Date	Chk'd by	Date	App'd by	Date
	MF	04/07/22	LS	04/07/22	LS	04/07/22

3 Applicable Design Standards

AS/NZS 1170.0:2002	Structural Design Actions – General principles
AS/NZS 1170.1-2002	Structural Design Actions – Permanent, imposed, and other actions
AS/NZS 1170.2-2011	Structural Design Actions – Wind actions
AS 1891.4:2009	Industrial fall-arrest systems and devices – selection, use and maintenance
AS 3995 -2009	Design of steel lattice masts & Masts (Section E1)
AS/NZS 3678 2011	Structural Steel – hot-rolled plates, floorplates and slabs
AS/NZS 3679.1 2016	Structural Steel – hot rolled bars and sections
AS 1163 2016	Cold-formed structural steel hollow sections
AS/NZS 4100 2020	Steel Structures
AS/NZS 1554.1 2014	Structural Steel Welding – Welding of Steel Structures
AS 3600 2018	Concrete Structures
	1



	Client / Project				Job No.		
	Telco Antennas / 2	22	2-055				
ļ	Section	Sheet No./Rev.					
	Structural Computation Report					7/1	
İ	Calc. by	Date	Chk'd by	Date	App'd by	Date	
	MF	04/07/22	LS	04/07/22	LS	04/07/22	

4 Design Actions

The following design parameters were adopted in the structural analysis and design of the steel tower and concrete footing

4.1 General Principals

Importance level

Design working life
 50 years

4.2 Permanent Actions

Deign loads based on AS1170.1

Self-weight of steel tower members and connections

Self-weight of concrete footing

Antenna details as tabled below

	ANTENNA SCHEDULE							
ID	SIZE / STYLE	BEARING	WEIGHT (kg)	ABOVE GROUND LEVEL (AGL)				
A1.06	600 DIAMETER PARABOLIC ANTENNA	102°	8	24m				
A2.06	600 DIAMETER PARABOLIC ANTENNA	112°	8	24m				
A3.06	600 DIAMETER PARABOLIC ANTENNA	334°	8	24m				
FUTURE	600 DIAMETER PARABOLIC ANTENNA	(TBC)	8	22m				
FUTURE	600 DIAMETER PARABOLIC ANTENNA	(TBC)	8	22m				

4.3 Imposed Actions

- Design loads based on AS1891.4 Industrial fall arrest systems and devices; part 4
- Single point anchorage 15kN applied at 5° from vertical to simulate real world conditions

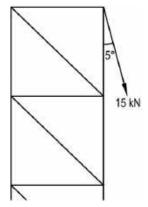


Figure 1 – Fall Arrest Load



Client / Project				Job No.		
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Section				Sheet No./Rev.		
Structural Computation Report					8 / 1	
Calc. by	App'd by	Date				
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Wind Actions

The following wind parameters were used based on AS3995 and AS1170.2:

Regions: C (refer map below for Australian wind regions)

Terrain category:

Terrain / height multiplier: 1.098

Wind directional multiplier mast / antennas: 0.9

Topographical Multiplier: 1.0

Climate Change Multiplier: 1.05

Shielding Multiplier: 1.0

- Lateral deflection to allow continuous communication is taken from AS3995 Appendix E1 which specifies a wind speed of 27m/s
- The ultimate wind speed and pressures (without the aerodynamic shape factors C_{fig}) are listed in the following table:

Region	V _R (m/s)	V _{des} (m/s)	P _w (kpa)
С	65.6	67.9	2.77

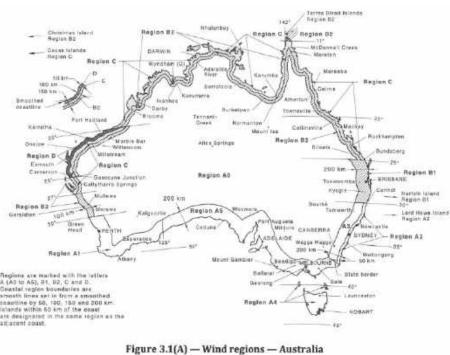


Figure 2 – AS1170.2 - Wind Regions - Australia



Client / Project				Job No.		
Telco Antennas / 24m Steel Tower at Bowen Orbital Spaceport				22-055		
Section				Sheet No./Rev.		
Structural Computation Report					9 / 1	
Calc. by	Date	Chk'd by	Date	App'd by	Date	
MF	04/07/22	LS	04/07/22	LS	04/07/22	

5 Tower Climbing Adequacy

Structural elements have been assessed for loads generated from climbing / working at heights. A single person load of 15kN is applied in accordance with Table 3.1 in AS1891.4.

AS1891.4 requires that signage be installed at the tower base describing which elements of the structure can be used for anchorage.

Any climber / worker at heights must be competent and qualified to do so.

Two (2) climbers / workers at heights can be supported by the tower with adherence to the following specifications.

- a. **ONLY** the members shown in purple in the generic diagram below can be used as a climbing / work at heights anchor point.
- b. The specified anchor points are limited to one (1) person at any time.
- c. Prior to anchoring to any element, visually check connections are complete and have no damage. Any defects should be reported to the manufacturer.
- d. Do not attach fall arrest devices to horizontal cross bracing.
- e. Any anchor point is limited to the tower leg above the brace connection points.

Generic Tower Anchor points

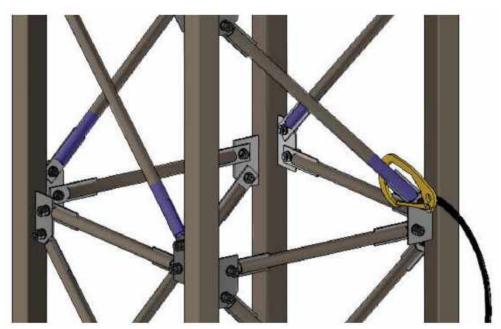


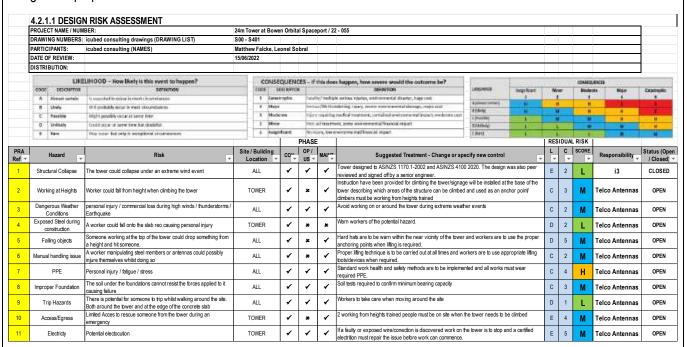
Figure 3 – Lanyard Hook must be placed around diagonal tower web above bracing connections only (purple shading)



Client / Project	•				
Telco Antennas /	22	2-055			
Section	Sheet No./Rev.				
Structural Computation Report				1	10 / 1
Calc. by	Date	Chk'd by	Date	App'd by	Date
MF	04/07/22	LS	04/07/22	LS	04/07/22

6 Safety In Design

Safety in Design (SID) is a statutory requirement on construction projects. In all jurisdictions within Australia there is a general requirement that the safety of a design is reviewed where non-standard construction techniques or novel building or structural designs are proposed.





Client / Project			Job No.		
Telco Antennas / 2	22-055				
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Section				Sheet No./Rev.	
Str	Structural Computation Report				11/1
Calc. by	Date	Chk'd by	Date	App'd by	Date
MF	04/07/22	LS	04/07/22	LS	04/07/22

7 Structural Analysis & Results

Below are the analysis and capacity results from the structural modelling for the 24m high, 1m face steel tower:

- The tower was analysed with the calculated ultimate and serviceability design loads
- Antenna loads were applied as a concentrated load at the specified AGL and bearing orientation as noted in section 4.2.

1m Face Lattice Tower –	24m high		
Wind region	С		
Material	Steel		
Tower leg			
- module A	89 x 6 SHS (G350)		
- module B, C and D	89 x 3.5 SHS (G350)		
Tower web	42.4 x 3.2 CHS (G250)		
leg centre to centre	1000 mm		
max tower design height	24 m		
No. of modules	4 x 6.0m		
Leg max design stress (Mpa)	171.27		
Grade 350 steel stress capacity (Mpa)	350		
Web max design stress (Mpa)	94.52		
Grade 250 steel stress capacity (Mpa)	250		

It can be seen from the table above that the stress capacity of the tower framing members are greater than the maximum design stress. Therefore, the steel tower is deemed structurally adequate.

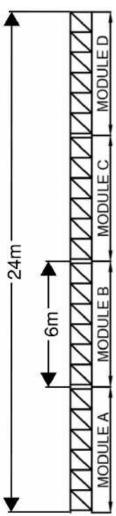


Figure 4 – Tower Layout



	Client / Project	Client / Project					
	Telco Antennas / 24m Steel Tower at Bowen Orbital Spaceport					2-055	
	Section				Sheet No./Rev.		
	Structural Computation Report				1	12 / 1	
	Calc. by	Date	Chk'd by	Date	App'd by	Date	
	MF	04/07/22	LS	04/07/22	LS	04/07/22	

7.1 Lateral Displacement

Tower lateral displacement at the top of the tower is tabled below

24m Tower at Bowen Orbital Spaceport	Lateral shift	Rotation
Lateral displacement at top of tower based on 27m/s wind	42.5mm	1.1°

Notes:

- Deflections for communication serviceability taken as 27m/s as per AS3995 appendix E1
- Actual deflections are subject to the antenna size, shape, orientation and placement on the tower.

7.2 Tower Base Reactions

From the structural analysis model, the table below provides the base reaction forces per leg to be supported by the footing system.

1m Face Lattice Tower – 24m high – Design Actions				
Location	Uplift	Shear	Bearing	Bending
Max tower base reaction ULS (kN)	498.28	14.97	509.76	-

7.3 Geotechnical Parameters

Geotechnical data of soil properties and conditions were obtained in the Site Classification & Geotechnical Recommendations report from CQ Soil Testing number CQ20875, dated 06/04/22. The results are shown below.

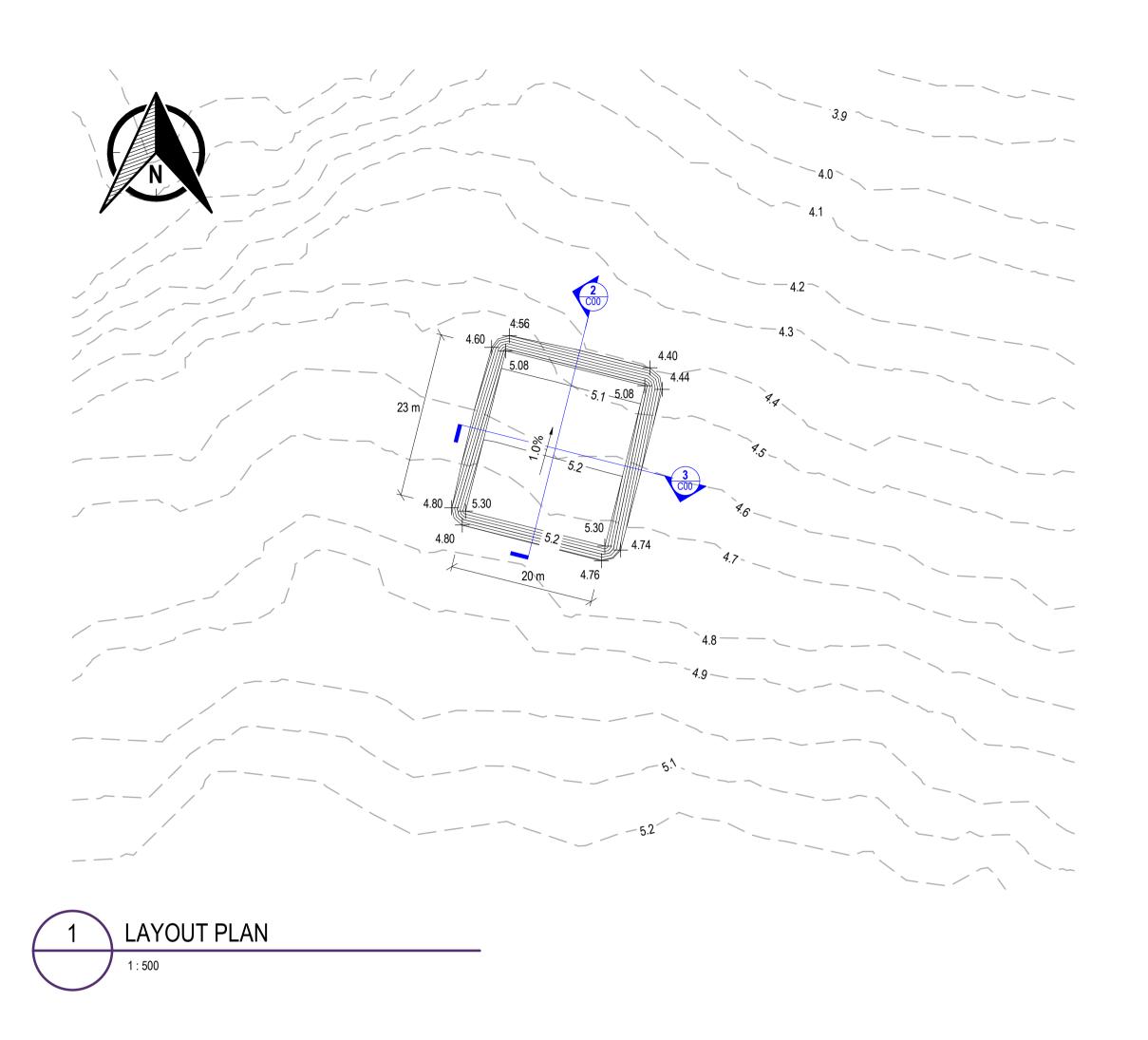
Soil Description & Assumed Design Parameters	
Soil type / description	silty clay / very stiff
Friction angle Φ	19°
Undrained shear strength Cu (kPa)	350 kPa(ULS)
Static Stress-Strain Es (MPa)	8-30

7.4 Footing Design

See the below capacity summary of the concrete footing.

Concrete footing Design Actions	Design Load	Design Capacity	Capacity Utilisation
Bearing	50.9 kPa	150 kPa	33.9 %
Sliding	51.42 kN	263.01 kN	19.6 %
Bending Moment	213.5 kN-m	481.4 kN-m	44.3 %
Shear	304.4 kN	522.6 kN	58.3 %
Punching Shear	509.76 kN	3078.6 kN	16.6 %
Minimum Steel	5313mm ²	6232mm ²	85.3%

As seen in the above table, the footing resistance against the design actions is satisfactory and therefore can be concluded that the footing system will be adequate for the conditions at Bowen Orbital Spaceport.



BULK EARTHWORKS NOTES

THE CONTRACTOR SHALL CARRY OUT FIELD DENSITY TESTING OF BULK FILLED AREAS INCLUDING BUILDING PLATFORMS AND REPLACED INSITU MATERIAL AS PER THE FOLLOWING REQUIREMENTS:

- 1. WORKS TO BE IN ACCORDANCE WITH AS 3798 2007.
- 2. TESTING TO BE IN ACCORDANCE WITH AS 1289 (RELEVANT PORTIONS).
- 3. SPECIFIC TESTING REQUIREMENTS
- LEVEL ONE TESTING TO BE PROVIDED BY GEOTECHNICAL TESTING AUTHORITY (GTA). - A DAILY GEOTECHNICAL SITE RECORD IS TO BE KEPT, REFER AS 3798, APPENDIX C
- 4. QUANTITY OF TESTING TO BE NOT LESS THAN ANY OF THE FOLLOWING - 1 TEST PER LAYER PER 2500sqm
- 1 TEST PER 500cub.m DISTRIBUTED EVENLY THROUGHOUT THE DEPTH AND AREA OF OF THE FILLING
- 3 TESTS PER VISIT BY THE GEOTECHNICAL TESTING AUTHORITY (GTA)
- 5. DEGREE OF COMPACTION TO BE AS FOLLOWS
- NSL TO 500mm BELOW FSL = 95% STANDARD COMPACTION - 500mm BELOW FSL TO FSL OR FPL = 100% STANDARD COMPACTION
- 6. MATERIAL MOISTURE CONTENT TO BE IN THE RANGE OF 85% TO 115% OF OPTIMAL MOISTURE CONTENT (OMC)
- 7. COMPACT SUBGRADE SURFACES IN CUT AREAS OF THE PLATFORM SHALL HAVE DRY DENSITY TESTING PERFORMED AT INTERVALS NOT LESS THAN 1 PER 2000sq.m
- 8. THE EXPENSE OF THE TESTING IS THE RESPONSIBILITY OF THE CONTRACTOR.

PAVEMENT NOTES

- 1. ALL PAVEMENTS ARE BASED ON A SOUND AND TRAFFICABLE SUBGRADE.
- WET AND/ OR SOFT AREAS FAILING THE SUBGRADE PROOF ROLL TEST MAY REQUIRE SOME FORM OF SUBGRADE IMPROVEMENT.
- 3. THE DESIGN ENGINEER SHALL BE CONSULTED TO ASSESS OPTIONS
- SUCH AS; LIME STABILISATION - GEOTEXTILE STRENGTHENING
 - COARSE ROCKFILL STRENGTHENING SUBGRADE REPLACEMENT
 - OR A COMBINATION OF ALL THESE OPTIONS.

GENERAL NOTES

- 1. THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL OTHER CONSULTANTS DRAWINGS AND SPECIFICATIONS. THE PROJECT SPECIFICATION SHALL TAKE PRECEDENCE OVER THESE GENERAL
- 2. BEFORE PROCEEDING WITH THE WORK ANY DISCREPANCIES IN THE CONTRACT DOCUMENTS SHALL BE REFERRED FOR DECISION TO THE DESIGN ENGINEER.
- 3. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT EDITIONS INCLUDING AMENDMENTS OF THE RELEVANT GLADSTONE REGIONAL COUNCIL STANDARDS, TMR STANDARDS, QLD CODES OF PRACTICE.
- 4. PROVIDE AS-CONSTRUCTED SURVEY FOR ALL WORKS.
- 5. THE SURVEY OF EXISTING SURFACE LEVELS HAS BEEN DERIVED FROM PUBLICLY AVAILABLE LIDAR AND MAY NOT BE 100% ACCURATE. CONFIRM EXISTING SURFACE LEVELS PRIOR TO CONSTRUCTION.
- 6. ACCESS RAMPS TO THE LAUNCH PAD TO BE DESIGNED BY OTHERS
- 7. AS PER GEOTECHNICAL REPORT GE 2211.1150, ACID SULPHATE HAS BEEN IDENTIFIED AND NEEDS TO BE TREATED APPROPRIATELY. REFER TO ACID SULPHATE TREATMENT PLAN.
- 8. CONCRETE SLAB DESIGNS FOR THE STORAGE TANKS ARE TO BE DESIGNED BY OTHERS.

HOLD POINT:

PROOF ROLL TO BE CONDUCTED ON THE SUBGRADE. CONTACT ENGINEER FOR AN INSPECTION 24 HOURS PRIOR TO THE REQUIRED INSPECTION TIME.

FOR A SUCCESSFUL PROOF ROLL INSPECTION: 1. NO SOFT SPOTS ARE TO BE FOUND - CONTRACTOR TO CONDUCT A PROOF ROLL ON SITE PRIOR TO CONTACTING ENGINEER TO ENSURE PAVEMENT WILL PASS. 2. THE PROOF ROLL IS ASSUMED TO BE CONDUCTED IN A SINGLE STAGE. IF A DIFFERENT APPROACH IS NEEDED, CONTACT THE ENGINEER.

> PLANS AND DOCUMENTS referred to in the SDA APPROVAL

SDA approval: APC2024/006

NOTES:

Client:

BULK EARTHWORKS ARE TO BE GRADED LINEARLY BETWEEN EARTHWORKS POINTS.

2. REFER SEDIMENT AND EROSION CONTROL NOTES AND DETAILS.

- -12.00— — — EXISTING SURFACE CONTOUR LABEL

BULK EARTHWORKS LEGEND

DESCRIPTION

— FINISHED PAD LEVEL

EXISTING SURFACE CONTOURS

BULK EARTHWORKS SPOT ELEVATIONS

icubed consulting

innovation ingenuity inspiration

Development Services | Engineering | Project Delivery

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Gilmour Space

All Orbits, All Planets®

https://www.gpsace.com

- 3. REFER EARTHWORKS PLAN FOR PAVEMENT ALLOWANCES.
- 4. TOLERANCES REFER CONTRACT DOCUMENTATION
- 5. SITE BOUNDARIES TO BE CONFIRMED BY SURVEY
- 6. SITE SERVICES PROVIDED BY SUBDIVISION DOCUMENTATION ONLY AND IT IS THE CONTRACTORS RESPONSIBILITY TO CONFIRM ALL LOCATIONS AND HEIGHTS BEFORE COMMENCING

LRE LAUNCH PAD EARTHWORK VOLUMES	
STRIP	166.0 cub.m
CUT 123.9 cub.m	
TYPE 2.5 GRAVEL	854.5 cub.m
TYPE 2.3 GRAVEL	106.3 cub.m

A PRELIMINARY 03.03.2023 L.K. REV DESCRIPTION DATE BY Status

PRELIMINARY NOT TO BE USED FOR CONSTRUCTION

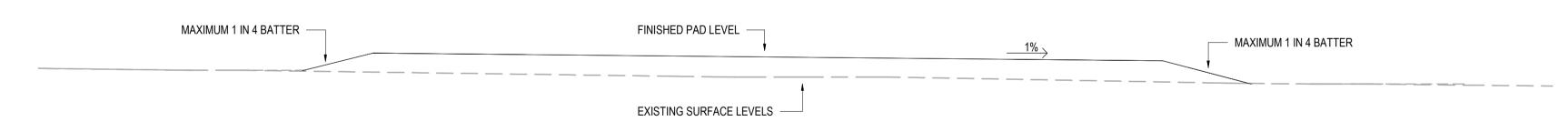
BOWEN ORBITAL SPACE PORT BOWEN QUEENSLAND

GILMORE SPACE TECH

LRE LAUNCH PAD EARTHWORKS PLAN

Drawn	Date	Chkd	Date
L.K.	03.03.2023	T.J.S	
Design	Date	Apprd	Date
L.K.		T.J.S	
Scale	A1	Certif	Date
As indicated		T.J.S	
Project No.		Dwg. No.	Rev
	21-307	C00	A

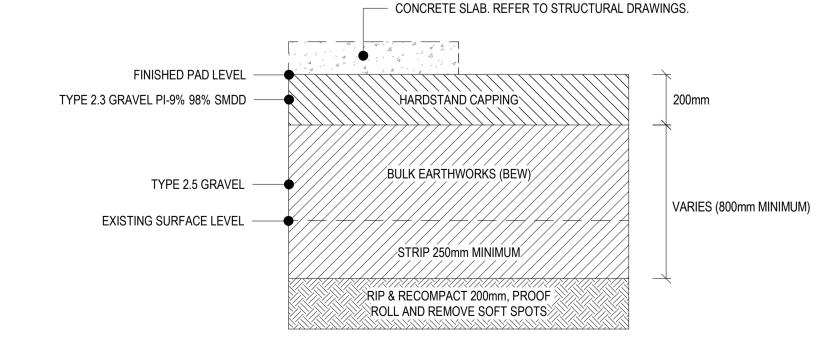
These drawings remain the property of i³ consulting pty. ltd. and must not be copied in any format without the written permission of the owners of the copyright.



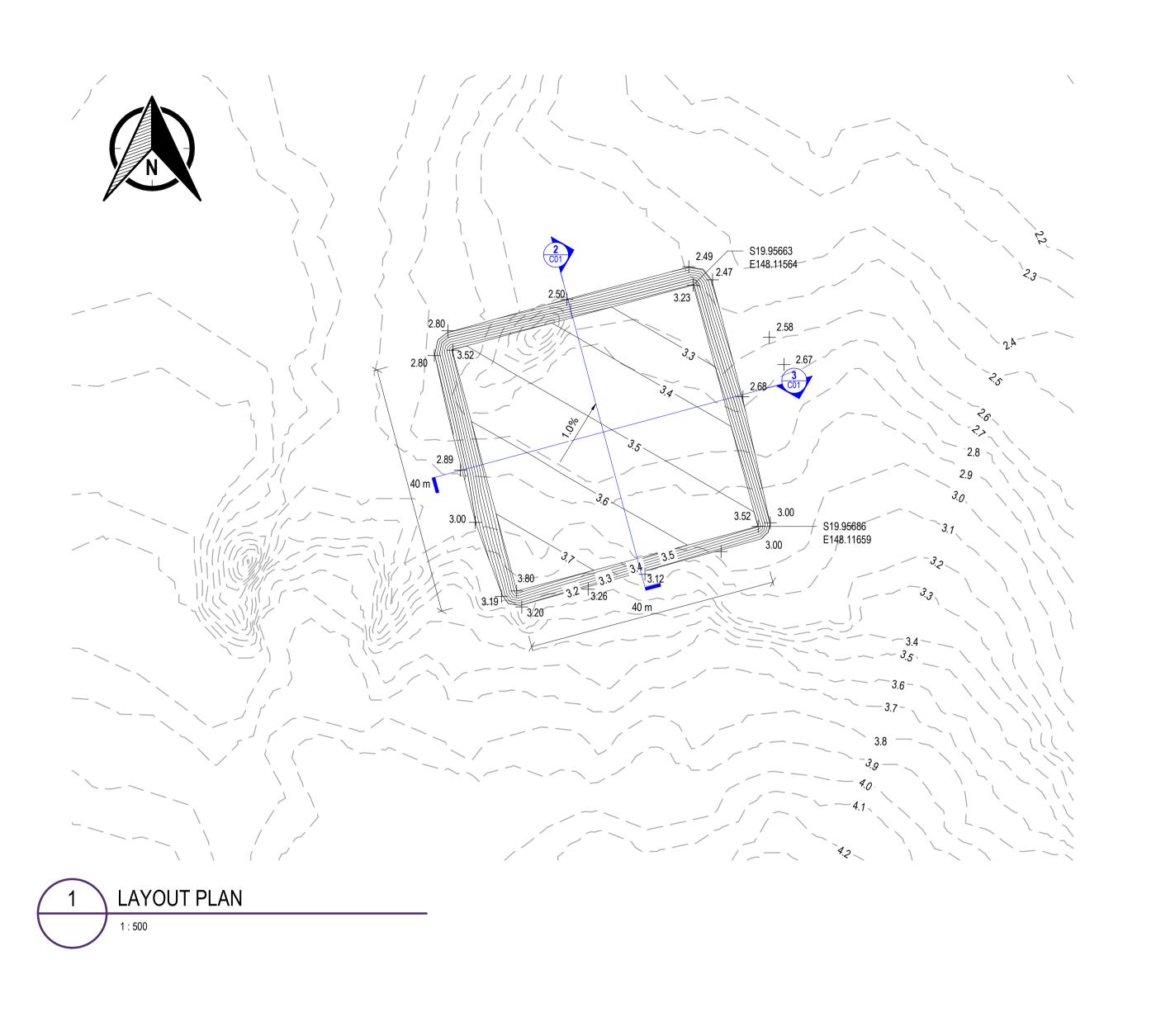
BULK EARTHWORKS SECTION 1

MAXIMUM 1 IN 4 BATTER -FINISHED PAD LEVEL -MAXIMUM 1 IN 4 BATTER EXISTING SURFACE LEVELS









GENERAL NOTES

- 1. THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL OTHER CONSULTANTS DRAWINGS AND SPECIFICATIONS. THE PROJECT SPECIFICATION SHALL TAKE PRECEDENCE OVER THESE GENERAL
- 2. BEFORE PROCEEDING WITH THE WORK ANY DISCREPANCIES IN THE CONTRACT DOCUMENTS SHALL BE REFERRED FOR DECISION TO THE DESIGN ENGINEER.
- 3. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT EDITIONS INCLUDING AMENDMENTS OF THE RELEVANT GLADSTONE REGIONAL COUNCIL STANDARDS, TMR STANDARDS, QLD CODES OF PRACTICE.
- 4. PROVIDE AS-CONSTRUCTED SURVEY FOR ALL WORKS.
- 5. THE SURVEY OF EXISTING SURFACE LEVELS HAS BEEN DERIVED FROM PUBLICLY AVAILABLE LIDAR AND MAY NOT BE 100% ACCURATE. CONFIRM EXISTING SURFACE LEVELS PRIOR TO CONSTRUCTION.
- 6. ACCESS RAMPS TO THE LAUNCH PAD TO BE DESIGNED BY OTHERS
- 7. AS PER GEOTECHNICAL REPORT GE_2211.1150, ACID SULPHATE HAS BEEN IDENTIFIED AND NEEDS TO BE TREATED APPROPRIATELY. REFER TO ACID SULPHATE TREATMENT PLAN.
- 8. CONCRETE SLAB DESIGNS FOR THE STORAGE TANKS ARE TO BE DESIGNED BY OTHERS.

PAVEMENT NOTES

MAXIMUM 1 IN 4 BATTER

- 1. ALL PAVEMENTS ARE BASED ON A SOUND AND TRAFFICABLE SUBGRADE.
- 2. WET AND/ OR SOFT AREAS FAILING THE SUBGRADE PROOF ROLL TEST MAY REQUIRE SOME FORM OF SUBGRADE IMPROVEMENT.
- 3. THE DESIGN ENGINEER SHALL BE CONSULTED TO ASSESS OPTIONS SUCH AS:
 - LIME STABILISATION
 - GEOTEXTILE STRENGTHENING
 - COARSE ROCKFILL STRENGTHENING
 - SUBGRADE REPLACEMENT
 - OR A COMBINATION OF ALL THESE OPTIONS.

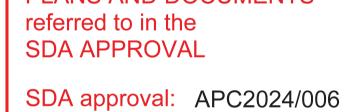
BULK EARTHWORKS NOTES

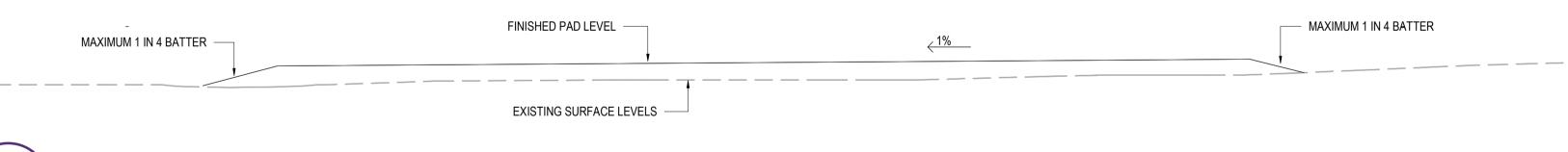
- 1. WORKS SHALL BE EXECUTED BY THE CONTRACTOR IN ACCORDANCE WITH AS3798 - 2007, 'GUIDELINES FOR COMMERCIAL AND RESIDENTIAL DEVELOPMENTS' AND SHALL BE SUPERVISED BE AN APPROVED GEOTECHNICAL TESTING AUTHORITY, (GTA) AS DEFINED IN THE CODE TO A LEVEL 2 STANDARD.
- 2. FOLLOWING INITIAL MOBILISATION AND PRIOR TO STRIPPING OF THE SITE, THE CONTRACTOR SHALL IMMEDIATELY UNDERTAKE THE EROSION AND SEDIMENT RUNOFF PREVENTION MEASURES.
- 3. FOLLOWING THE STRIPPING OF THE UPPER ORGANIC SOIL, THE SITE SHALL BE PROOF ROLLED TO IDENTIFY ANY SOFT SPOTS OR FURTHER SIGNS OF UNSUITABLE MATERIAL. ANY SUCH AREAS SHALL BE RECTIFIED PRIOR TO COMMENCEMENT OF FILLING OPERATIONS.
- 4. FILLING SHALL BE COMPLETED IN ACCORDANCE WITH THE FOLLOWING REQUIREMENTS
- WORKS TO BE IN ACCORDANCE WITH AS3798 2007 - DEGREE OF COMPACTION TO BE AS FOLLOWS
 - SUBGRADE COURSES LESS THAN 300mm BELOW DESIGN SURFACE LEVEL 98% SMDD.
 - SUBGRADE COURSES 300mm OR MORE BELOW DESIGN SURFACE LEVEL 95% SMDD.
 - SUBBASE COURSES 98% SMDD
- BASE COURSES 98% SMDD - MATERIAL MOISUTRE CONTENT TO BE IN THE RANGE OF $\pm 3\%$ OF OPTIMAL MOISTURE CONTENT (OMC)
- THE EXPENSE OF TESTING IS ON THE CONTRACTOR
- 5. IT IS THE CONTRACTORS RESPONSIBILITY TO PROTECT THE SITE AND SURROUNDING AREAS FROM DAMAGE RESULTING FROM STORMWATER RUNOFF. TEMPORARY DIVERSION DRAINS AND OR OTHER DRAINAGE CONTROL DEVICES ARE TO BE IMPLEMENTED BY THE CONTRACTOR DURING CONSTRUCTION TO MINIMISE THE EFFECTS OF WEATHER. NO EXTENSIONS OF TIME WILL BE GRANTED SHOULD DAMAGE TO THE WORK AND SURROUNDING AREAS RESULT FROM THE CONTRACTORS NEGLIGENCE IN NOT PROVIDING ADEQUATE PROTECTION.
- 6. EXCESS SPOIL MATERIAL GENERATED DURING CONSTRUCTION SHALL BE REMOVED FROM THE SITE BY THE CONTRACTOR.

PROOF ROLL TO BE CONDUCTED ON THE SUBGRADE. CONTACT ENGINEER FOR AN INSPECTION 24 HOURS PRIOR TO THE REQUIRED INSPECTION TIME.

FOR A SUCCESSFUL PROOF ROLL INSPECTION: 1. NO SOFT SPOTS ARE TO BE FOUND - CONTRACTOR TO CONDUCT A PROOF ROLL ON SITE PRIOR TO CONTACTING ENGINEER TO ENSURE PAVEMENT WILL PASS. 2. THE PROOF ROLL IS ASSUMED TO BE CONDUCTED IN A SINGLE STAGE. IF A DIFFERENT APPROACH IS NEEDED, CONTACT THE ENGINEER.

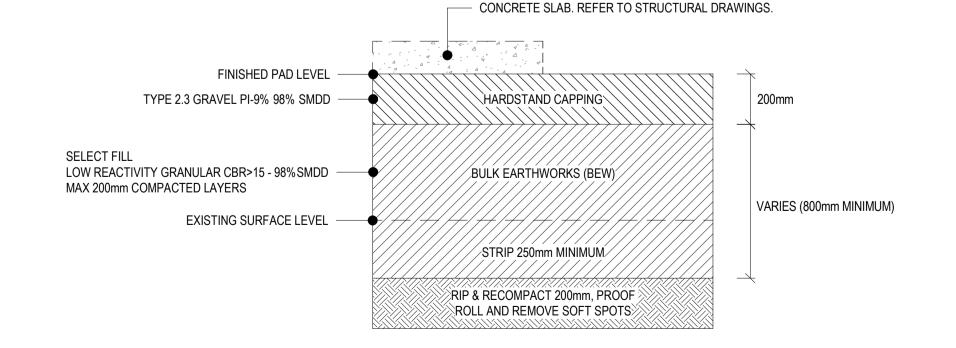
PLANS AND DOCUMENTS







MAXIMUM 1 IN 4 BATTER



EXISTING SURFACE LEVELS -**BULK EARTHWORKS SECTION 4**

FINISHED PAD LEVEL

TYPICAL PAD PAVEMENT DETAILS



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Client:

BULK EARTHWORKS LEGEND

DESCRIPTION — FINISHED PAD LEVEL - - - - EXISTING SURFACE CONTOURS - - -12.00- - EXISTING SURFACE CONTOUR LABEL BULK EARTHWORKS SPOT ELEVATIONS

- BULK EARTHWORKS ARE TO BE GRADED LINEARLY BETWEEN EARTHWORKS POINTS.
- 2. REFER SEDIMENT AND EROSION CONTROL NOTES AND DETAILS.
- 3. REFER EARTHWORKS PLAN FOR PAVEMENT ALLOWANCES.
- 4. TOLERANCES REFER CONTRACT DOCUMENTATION 5. SITE BOUNDARIES TO BE CONFIRMED BY SURVEY
- 6. SITE SERVICES PROVIDED BY SUBDIVISION DOCUMENTATION ONLY AND IT IS THE CONTRACTORS RESPONSIBILITY TO CONFIRM ALL LOCATIONS AND HEIGHTS BEFORE COMMENCING

SIRIUS LAUNCH PAD EARTHWORK VOLUMES	
STRIP	525.9 cub.m
CUT 0 cub.m	
SELECT FILL 1478 cub.m	
TYPE 2.3 GRAVEL 346.112 cub.m	

1	PAD AMENDED	29.02.2024	1.J.S.
0	FOR CONSTRUCTION	21.04.2023	P.J.M.
Α	PRELIMINARY	03.03.2023	L.K.
REV	DESCRIPTION	DATE	BY

FOR CONSTRUCTION

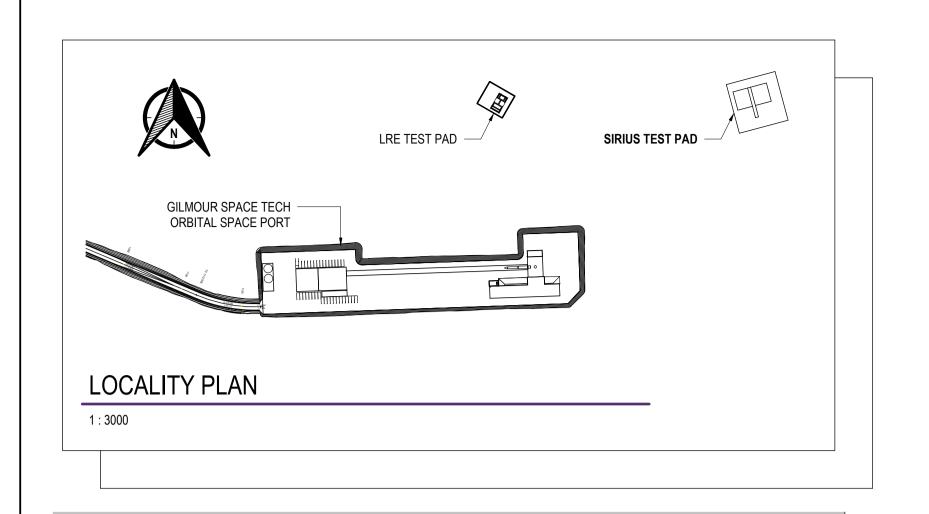
BOWEN ORBITAL SPACE PORT BOWEN QUEENSLAND

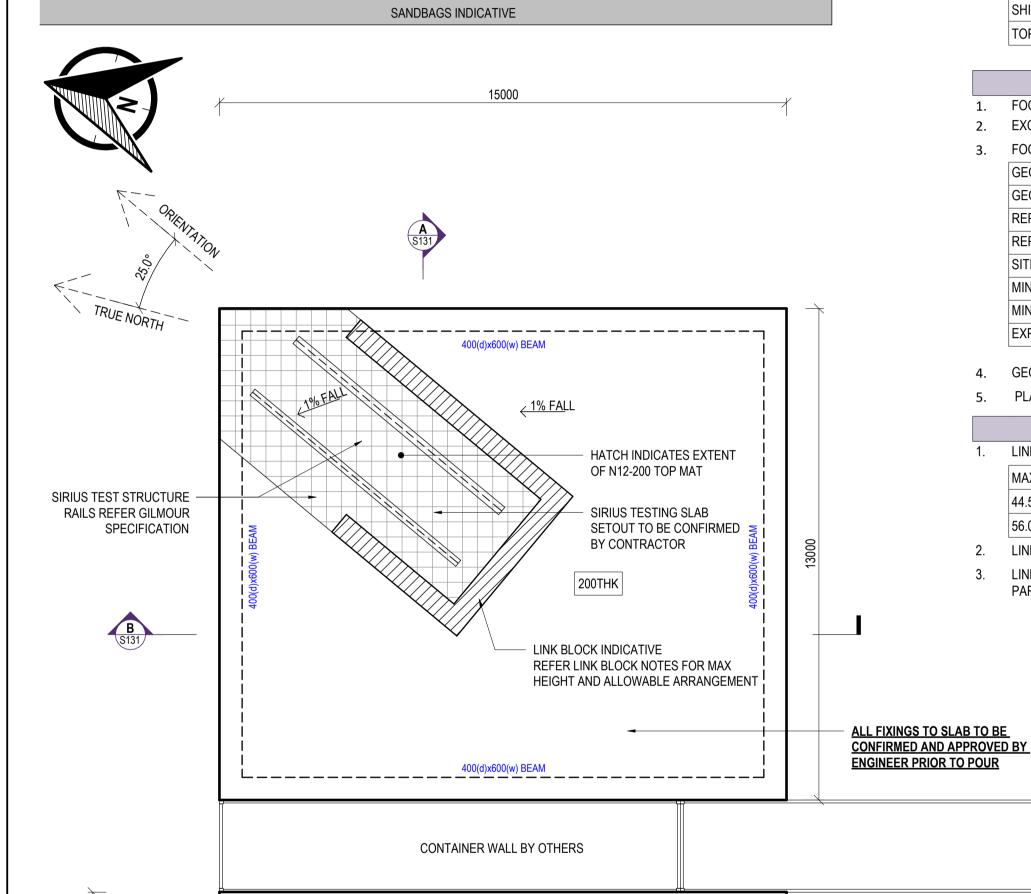
GILMORE SPACE TECH

SIRUIS LAUNCH PAD EARTHWORKS PLAN

	T.J.S Dwg. No.	Rev
	T.J.S	
A1	Certif	Date
	T.J.S	
Date	Apprd	Date
03.03.2023	T.J.S	
Date	Chkd	Date
	03.03.2023	03.03.2023 T.J.S Date Apprd

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300(d)x400(w) BEAM

< 1% FALL

200THK

DESIGN CRITERIA GENERAL PRINCIPLES HAVE BEEN ASSESSED IN ACCORDANCE WITH AS1170.0 IMPORTANCE LEVEL IL2 DESIGN LIFE 50 YEARS PERMANENT ACTIONS HAVE BEEN ASSESSED IN ACCORDANCE WITH AS1170.1: LINK BLOCK 3m HIGH SELF-WEIGHT 20,000L ISO TANK 5t DRY, 30t WET HTP BUND 26kL STORAGE CYLINDER MAN PACK 16kN POINT LOAD VARIOUS STORAGE TANKS MAX 5t IMPOSED ACTIONS HAVE BEEN ASSESSED IN ACCORDANCE WITH AS1170.1: SIRIUS THRUST LOAD 5t SELF WEIGHT 180kN THRUST DIRECTION 15kN LATERAL SHOCK 45kN OVERTURN SHOCK VEHICLE LOADING 2.5t FORKLIFT FOOT TRAFFIC WIND LOADS HAVE BEEN ASSESSED IN ACCORDANCE WITH AS1170.2: REGION TERRAIN CATEGORY TC2 WIND DIRECTION MULTIPLIER 0.9 SHIELDING MULTIPLIER 1.0

FOUNDATION NOTES

1.0

- FOOTINGS SHALL BE PLACED CENTRALLY UNDER COLUMNS UNLESS NOTED OTHERWISE.
- EXCAVATIONS SHALL BE KEPT FREE OF PONDED WATER BEFORE PLACING CONCRETE. FOOTINGS HAVE BEEN DESIGNED BASED ON THE FOLLOWING GEOTECHNICAL REPORT

TOPOGRAPHICAL MULTIPLIER

ROLLOVER BUND BY OTHERS

FIXINGS TO BE CONFIRMED

ALL FIXINGS TO SLAB TO BE CONFIRMED AND APPROVED BY

ENGINEER PRIOR TO POUR

ONLY FORKLIFT VEHICLE LOAD

HAS BEEN ALLOWED ON SLAB.

- **GEOTECHNICAL DETAILS** GEOTECHNICAL REPORT BY GROUND ENVIRONMENTS PTY LTD REPORT JOB NUMBER GE_2211.1150 REPORT DATE 11.01.2023 SITE CLASSIFICATION MIN. BEARING CAPACITY (ULS) 200kPa MIN PILE SKIN FRICTION (ULS) N/A **EXPECTED FOUNDING MATERIAL** CONTROLLED FILL
- GEOTECHNICAL ENGINEER TO CONFIRM FOUNDATION DESIGN PARAMETERS PRIOR TO PLACING CONCRETE
- PLACE BLINDING CONCRETE TO BASE OF FOOTING EXCAVATION TO ACHIEVE DESIGN AND / OR UNIFORM BEARING MATERIAL

LINK BLOCK NOTES

LINK BLOCK MAX HEIGHT DURING WIND EVENTS:	
MAXIMUM WIND GUST SPEED	MAX WALL HEIGHT
44.5m/s	1.8m (DURING SERVICE)
56.0m/s	1.2m

- LINK BLOCK WALLS SHALL BE UNSTACKED PRIOR TO STORM EVENTS AND OUT OF SERVICE.
- LINK BLOCK WALLS CAN BE RELOCATED AND ARRANGED TO SUIT SERVICE OPERATIONS. DO NOT ARRANGE LINK BLOCK WALLS IN PARALLEL ROWS AT LESS THAN 1m CENTRES

SLAB ON GROUND NOTES

- SUBGRADE TO BE CONSTRUCTED AS PER EARTHWORKS NOTES. SLAB TO BE PLACED ON 50mm BEDDING SAND WITH WATERPROOF MEMBRANE AS PER CONRETE NOTES.
- 3. PROVIDE 2N12 TRIMMER BARS 1000 LONG TO ALL RE-ENTRANT
- 4. SLAB TO HAVE A NOMINAL 1% FALL FOR SURFACE DRAINAGE OF

600 LAP FOOTING REO REFER DETAILS LIGS - REFER DETAILS CORNER BARS SIZE TO MATCH BEAM REINFORCEMENT TOP AND BOTTOM 600 LAP POSITION AS SHOWN CORNER STRIP FOOTING WITH REINFORCEMENT BARS

TYPICAL FOOTING CORNER DETAIL

NTS

FOOTING CORNER DETAILS

CONCRETE NOTES

- ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS 3600 & AS 1379.
- CONCRETE QUALITY AND CLEAR CONCRETE COVER TO REINFORCEMENT (OUTSIDE OF STIRRUPS AND TIES) SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:

ELEMENT	CONCRETE GRADE	CLEAR COVER	SLUMP
BEAMS	N32	30 TOP COVER 50 BTM COVER 50 SIDE COVER	SLUMP 90 PLUS 20 MINUS 20
SLABS	REFER SLAB ON GROUND SCHEDULE ON DRAWINGS	30 TOP COVER 50 BTM COVER 50 SIDE COVER	SLUMP 90 PLUS 20 MINUS 20

- CONCRETE MIX TO HAVE A MAXIMUM AGGREGATE SIZE OF 20mm.
- SPECIFIED STRUCTURAL THICKNESSES FOR CONCRETE ELEMENTS DO NOT INCLUDE THICKNESS OF APPLIED FINISHES.
- CONSTRUCTION JOINTS OR POUR BREAKS WHERE NOT SHOWN ON PLANS OR DETAILS SHALL BE LOCATED AND FORMED TO THE APPROVAL OF THE ENGINEER.
- NO PENETRATIONS, RECESSES OR CHASES OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE MADE IN CONCRETE MEMBERS WITHOUT APPROVAL OF THE ENGINEER.
- AT PENETRATIONS IN SLABS UNLESS OTHERWISE DETAILED REINFORCEMENT MUST NOT BE CUT BUT SHALL BE GATHERED EQUALLY TO EACH SIDE OF PENETRATION AND EXTRA REINFORCEMENT PROVIDED BETWEEN THE PENETRATIONS AS DIRECTED BY THE ENGINEER.
- REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY. IT IS NOT NECESSARILY SHOWN IN TRUE PROJECTION.
- PROVIDE THE ENGINEER WITH 48 HOURS NOTICE OF REINFORCEMENT BEING READY FOR INSPECTION. NO CONCRETE IS TO BE POURED WITHOUT THE APPROVAL OF THE ENGINEER. THIS IS TO BE CONFIRMED AT A LATER DATE.
- 10. SPLICES IN REINFORCEMENT SHALL BE MADE ONLY IN THE POSITIONS SHOWN. WHERE LAP LENGTH IS NOT SHOWN IT SHALL BE SUFFICIENT TO DEVELOP THE FULL STRENGTH OF THE REINFORCEMENT. THE FOLLOWING MINIMUM SPLICE LENGTHS SHALL BE USED UNLESS NOTED OTHERWISE:

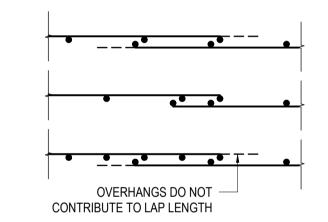
THE FOLLOWING MINIMUM BAR SPLICE LENGTHS ARE APPLICABLE FOR fc >= 25MPa TO 65MPa.

A - FOR VERTICAL OR HORIZONTAL BARS WITH LESS THAN 300mm OF CONCRETE CAST BELOW			
BAR DIAMETER	LAP LENGTH (mm)	BAR DIAMETER	LAP LENGTH (mm)
N10	360	N24	1165
N12	440	N28	1485
N16	605	N32	1835
N20	865	N36	2210

B - FOR HORIZONTAL BARS GREATER THAN OR EQUAL TO 300mm OF CONCRETE CAST BELOW

BAR DIAMETER	LAP LENGTH (mm)	BAR DIAMETER	LAP LENGTH (mm)
N10	470	N24	1550
N12	570	N28	1960
N16	785	N32	2385
N20	1125	N36	2870

11. SUPPLY AND LAY FABRIC IN FLAT SHEETS AT SPLICES. FABRIC SHALL BE LAPPED AS SPECIFIED IN AS 3600. STEEL MESH SHOULD BE LAPPED AS SHOWN BELOW IE THE TWO OUTERMOST CROSS-WIRES OF ONE SHEET SHOUD OVERLAP THE TWO OUTERMOST WIRES AT THE OTHER.



- MAINTAIN 30 MIN. COVER TO REINFORCEMENT AT SERVICES AND CAST IN ITEMS THROUGH SLABS AND FOOTINGS. ALL SERVICES THROUGH FOOTING TO BE WRAPPED WITH COMPRESSIBLE MATERIAL MINIMUM 10mm ABLEFLEX OR SIMILAR.
- WELDING OF REINFORCEMENT WILL ONLY BE PERMITTED WITH THE PRIOR APPROVAL OF THE ENGINEER.
- 14. REINFORCEMENT MUST NOT BE CONTINUOUS THROUGH CONTRACTION JOINTS.
- 15. PLACE SUFFICIENT SUPPORT HURDLES/CHAIRS UNDER MAIN BOTTOM REINFORCING BARS AND TOP CROSS BARS IN SLABS AND FOOTINGS TO ALLOW THEM TO BE SUPPORTED IN THEIR CORRECT POSITIONS AND ALIGNMENT DURING CONCRETING (NOT GREATER THAN 900mm CENTRES UNLESS SHOWN OTHERWISE).

16. REINFORCEMENT SYMBOLS:

SYMBOL	SPECIFICATION	
N	GRADE D500N DEFORMED BAR	
R	STRUCTURAL GRADE ROUND BAR	
RF AND SL HARD DRAWN SHEET RIBBED WIRE REINFORCING FABRIC		
THE NUMBER FOLLOWING THESE SYMBOLS IS THE BAR DIAMETER IN MILLIMETERS.		

- 17. SLABS TO BE CURED BY COVERING WITH 0.20mm BLACK POLYETHYLENE SHEETING AND KEPT MOIST FOR 7 DAYS MINIMUM OR APPROVED ALTERNATIVE CURING METHOD.
- 18. NO CONCRETE TO BE POURED WHEN SITE TEMPERATURE EXCEEDS 35°C OR FALLS BELOW 5°C.

STRUCTURAL GROUT NOTES

- GROUT IS TO BE HIGH STRENGTH AND NON-SHRINK.
- ALL GROUT TO BE CEMENTITIOUS UNLESS NOTED OTHERWISE.
- MINIMUM 20MPa COMPRESSIVE STRENGTH FLOWABLE GROUT TO BE APPROVED BY ICUBED
- WHERE ALTERNATIVE GROUT PRODUCTS ARE USED THE FOLLOWING PROPERTIES ARE TO BE ACHIEVED:

COMPRESSIVE STRENGTH (AS1478.2:2005)	55MPa AT 1 DAYS	
	80MPa AT 7 DAYS	
FLEXURAL STRENGTH (MODULUS OF RUPTURE AS1012.2.11:2000)	11.4MPa AT 7 DAYS	
INDIRECT TENSII E STRENGTH (AS1012 2 10:2000)	5 1MPa AT 7 DAYS	

- TOP OF CONCRETE TO HAVE A CONCRETE SURFACE PROFILE (CSP) OF MIN 2.0 (GRIND AND WATER BLAST SURFACE) IN ACCORDANCE WITH ICRI TECHNICAL GUIDELINE NO. 310.2R2013.
- ALL GROUT PRODUCTS ARE TO BE STORED, HANDLED AND PLACED STRICTLY IN ACCORDANCE WITH MANUFACTURERS CURRENT TECHNICAL DATA SHEET AND INSTRUCTIONS.

COATING NOTES

- THE FOLLOWING COATING REQUIREMENTS HAVE BEEN DETERMINED BY AS4312. ALL FABRICATED STRUCTURAL STEELWORK TO BE PREPARED WITH ABRASIVE BLAST CLEANING CLASS 2.5.
- SURFACES REQUIRING ONSITE TREATMENT ARE TO BE PRIMED IN ACCORDANCE WITH SURFACE TREAMENT
- MANUFACTURERS REQUIREMENTS.
- 4. COATINGS HAVE BEEN SPECIFIED BASED ON THE FOLLOWING:

CORROSIVITY CATERGORY	C3
STEEL CORROSION RATE µm/y	25 - 50
TYPICAL ENVIRONMENT	COASTAL/INDUSTRIAL
DURABILITY CLASS	LONG TERM
LIFE TO FIRST MAINTENANCE	10 YEARS TBC BY GILMOUR SPACE TECH

SURFACE TREATMENT:	
EXPOSED STRUCTURAL STEEL	HDG390/ILG100/ZB100
INTERNAL STRUCTURAL STEEL	HDG390/ILG100/ZB100

PROJECT TEAM

Consulting Engineer:



consulting pty ltd engineering consultants innovation, ingenuity, inspiration

07 3870 8888



CONSTRUCTION NOTES:

- REFER GENERAL CONSTRUCTION NOTES ON DWG S01 REFER DRAWING ABBREVIATIONS ON S01
- REFER EARTHWORKS NOTES ON DWG S01

PLANS AND DOCUMENTS referred to in the SDA APPROVAL



SDA approval: APC2024/006

REV	DESCRIPTION	DATE	BY
A	FOR COMMENT	10.03.2023	J.A.W.
В	FOR APPROVAL	03.05.2023	J.A.W.
0	FOR CONSTRUCTION	17.05.2023	J.A.W.
1	FOR CONSTRUCTION	26.02.2024	J.A.W.

FOR CONSTRUCTION

BOWEN ORBITAL SPACE PORT SIRIUS TEST PAD **BOWEN QUEENSLAND**

GILMOUR SPACE TECH

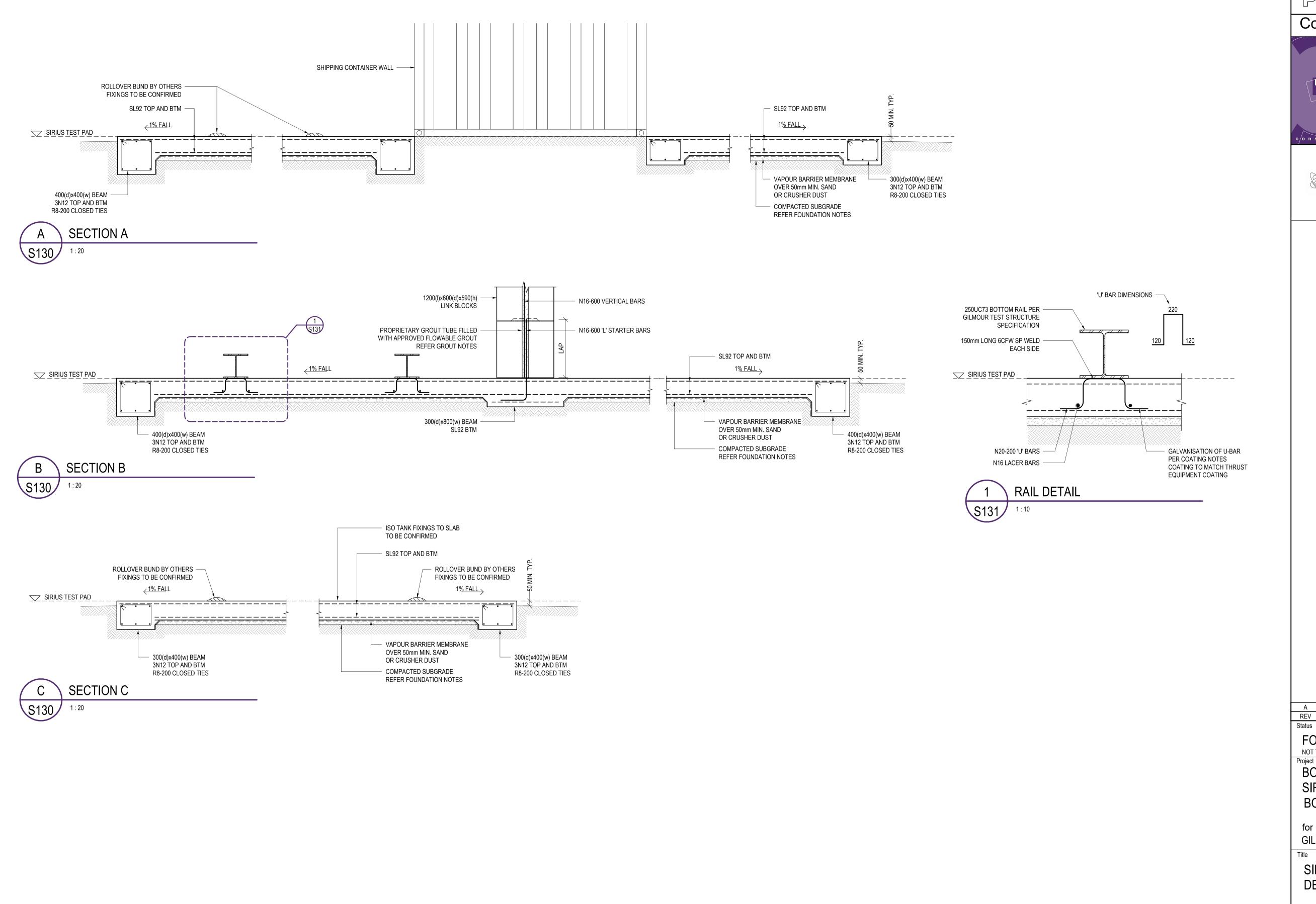
SIRIUS TEST PAD SLAB PLAN

Drawn	Date	Chkd	Date
JAW		RM	
Design	Date	Apprd	Date
LV		RM	
Scale	A1	Certif	Date
As indicated	d		
Project No.		Dwg. No.	Rev
	21-307	S130	1

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1:100

SIRIUS PAD SLAB PLAN



PLANS AND DOCUMENTS referred to in the SDA APPROVAL

SDA approval: APC2024/006

Consulting Engineer:



consulting pty Itd engineering consultants innovation, ingenuity, inspiration

Milton, Qld 4064 Suite, 5 Gardner Close www.icubed.com.au mail@icubed.com.au

A FOR COMMENT 10.03.2023 J.A.W. REV DESCRIPTION DATE BY FOR COMMENT

NOT TO BE USED FOR CONSTRUCTION

BOWEN ORBITAL SPACE PORT SIRIUS TEST PAD **BOWEN QUEENSLAND**

GILMOUR SPACE TECH

SIRIUS TEST PAD CONCRETE **DETAILS**

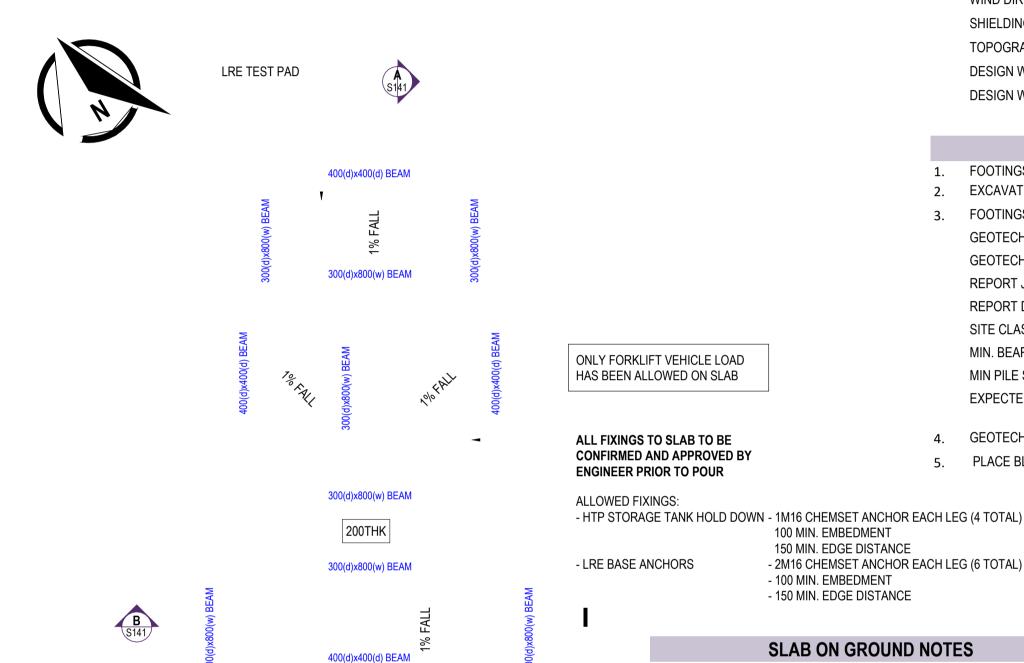
Chkd Date Drawn Date Design LV Scale Certif As indicated Project No. Dwg. No. 21-307 S131

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LOCALITY PLAN

1:3000



50 YEARS DESIGN LIFE 2. PERMANENT ACTIONS HAVE BEEN ASSESSED IN ACCORDANCE WITH AS1170.1: LINK BLOCK 3m HIGH SELF-WEIGHT 16kN POINT LOAD CYLINDER MAN PACK VARIOUS STORAGE TANKS 26kL STORAGE HTP STORAGE TANK 7t EMPTY, 13t FULL IMPOSED ACTIONS HAVE BEEN ASSESSED IN ACCORDANCE WITH AS1170.1: LRE THRUST LOAD 1.5t SELF WEIGHT 60kN THRUST DIRECTION 2.5t FORKLIFT VEHICLE LOADING 5kPA FOOT TRAFFIC 4. WIND LOADS HAVE BEEN ASSESSED IN ACCORDANCE WITH AS1170.2: REGION С TC2 TERRAIN CATEGORY WIND DIRECTION MULTIPLIER 0.9 SHIELDING MULTIPLIER

GENERAL PRINCIPLES HAVE BEEN ASSESSED IN ACCORDANCE WITH AS1170.0:

FOUNDATION NOTES

1.0

55.90m/s

1.875kPa

DESIGN CRITERIA

IL2

- FOOTINGS SHALL BE PLACED CENTRALLY UNDER COLUMNS UNLESS NOTED OTHERWISE.
- EXCAVATIONS SHALL BE KEPT FREE OF PONDED WATER BEFORE PLACING CONCRETE.
- 3. FOOTINGS HAVE BEEN DESIGNED BASED ON THE FOLLOWING GEOTECHNICAL REPORT:

GEOTECHNICAL DETAILS GEOTECHNICAL REPORT BY

EXPECTED FOUNDING MATERIAL

TOPOGRAPHICAL MULTIPLIER

DESIGN WIND PRESSURE (ULS)

DESIGN WIND SPEED (ULS)

IMPORTANCE LEVEL

GROUND ENVIRONMENTS PTY LTD GE_2211.1150 REPORT JOB NUMBER REPORT DATE 11.01.2023 SITE CLASSIFICATION MIN. BEARING CAPACITY (ULS) 200kPa MIN PILE SKIN FRICTION (ULS) N/A

- 4. GEOTECHNICAL ENGINEER TO CONFIRM FOUNDATION DESIGN PARAMETERS PRIOR TO PLACING CONCRETE.
- 5. PLACE BLINDING CONCRETE TO BASE OF FOOTING EXCAVATION TO ACHIEVE DESIGN AND / OR UNIFORM BEARING MATERIAL

CONTROLLED FILL

100 MIN. EMBEDMENT 150 MIN. EDGE DISTANCE - 2M16 CHEMSET ANCHOR EACH LEG (6 TOTAL) - 100 MIN. EMBEDMENT - 150 MIN. EDGE DISTANCE

SLAB ON GROUND NOTES

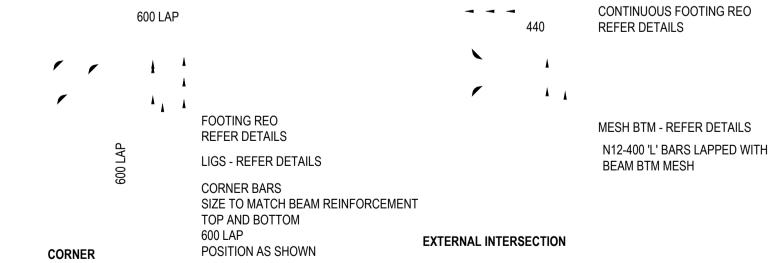
- SUBGRADE TO BE CONSTRUCTED AS PER EARTHWORKS NOTES.
- SLAB TO BE PLACED ON 50mm BEDDING SAND WITH
- WATERPROOF MEMBRANE AS PER CONRETE NOTES.
- PROVIDE 2N12 TRIMMER BARS 1000 LONG TO ALL RE-ENTRANT CORNERS.
- SLAB TO HAVE A NOMINAL 1% FALL FOR SURFACE DRAINAGE OF

SIRIUS PAD SLAB PLAN

1:100

PLANS AND DOCUMENTS referred to in the SDA APPROVAL

SDA approval: APC2024/006



STRIP FOOTING WITH REINFORCEMENT BARS

STRIP FOOTING WITH REINFORCEMENT BARS

TYPICAL FOOTING CORNER DETAIL TYPICAL FOOTING INTERSECTION DETAIL

FOOTING CORNER DETAILS

CONCRETE NOTES

ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS 3600 & AS 1379. CONCRETE QUALITY AND CLEAR CONCRETE COVER TO REINFORCEMENT (OUTSIDE OF STIRRUPS AND TIES) SHALL BE AS

ELEMENT SLUMP CONCRETE GRADE **CLEAR COVER** 30 TOP COVER SLUMP 90 **BEAMS** N32 50 BTM COVER PLUS 20 MINUS 20 50 SIDE COVER SLUMP 90 30 TOP COVER REFER SLAB ON GROUND **SLABS** 50 BTM COVER PLUS 20 SCHEDULE ON DRAWINGS 50 SIDE COVER MINUS 20

CONCRETE MIX TO HAVE A MAXIMUM AGGREGATE SIZE OF 20mm.

FOLLOWS UNLESS NOTED OTHERWISE:

- SPECIFIED STRUCTURAL THICKNESSES FOR CONCRETE ELEMENTS DO NOT INCLUDE THICKNESS OF APPLIED FINISHES. CONSTRUCTION JOINTS OR POUR BREAKS WHERE NOT SHOWN ON PLANS OR DETAILS SHALL BE LOCATED AND FORMED TO THE APPROVAL OF THE ENGINEER.
- NO PENETRATIONS, RECESSES OR CHASES OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE MADE IN CONCRETE MEMBERS WITHOUT APPROVAL OF THE ENGINEER.
- 7. AT PENETRATIONS IN SLABS UNLESS OTHERWISE DETAILED REINFORCEMENT MUST NOT BE CUT BUT SHALL BE GATHERED EQUALLY TO EACH SIDE OF PENETRATION AND EXTRA REINFORCEMENT PROVIDED BETWEEN THE PENETRATIONS AS DIRECTED BY THE ENGINEER.
- REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY. IT IS NOT NECESSARILY SHOWN IN TRUE PROJECTION.
- PROVIDE THE ENGINEER WITH 48 HOURS NOTICE OF REINFORCEMENT BEING READY FOR INSPECTION. NO CONCRETE IS TO BE POURED WITHOUT THE APPROVAL OF THE ENGINEER. THIS IS TO BE CONFIRMED AT A LATER DATE.
- 10. SPLICES IN REINFORCEMENT SHALL BE MADE ONLY IN THE POSITIONS SHOWN. WHERE LAP LENGTH IS NOT SHOWN IT SHALL BE SUFFICIENT TO DEVELOP THE FULL STRENGTH OF THE REINFORCEMENT. THE FOLLOWING MINIMUM SPLICE LENGTHS SHALL BE USED UNLESS NOTED OTHERWISE:

THE FOLLOWING MINIMUM BAR SPLICE LENGTHS ARE APPLICABLE FOR fc >= 25MPa TO 65MPa.

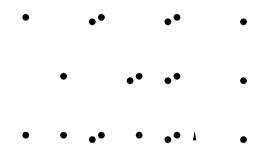
A - FOR VERTICAL OR HORIZONTAL BARS WITH LESS THAN 300mm OF CONCRETE CAST BELOW

BAR DIAMETER	LAP LENGTH (mm)	BAR DIAMETER	LAP LENGTH (mm)
N10	360	N24	1165
N12	440	N28	1485
N16	605	N32	1835
N20	865	N36	2210

B - FOR HORIZONTAL BARS GREATER THAN OR EQUAL TO 300mm OF CONCRETE CAST BELOW

BAR DIAMETER	LAP LENGTH (mm)	BAR DIAMETER	LAP LENGTH (mm)
N10	470	N24	1550
N12	570	N28	1960
N16	785	N32	2385
N20	1125	N36	2870

11. SUPPLY AND LAY FABRIC IN FLAT SHEETS AT SPLICES. FABRIC SHALL BE LAPPED AS SPECIFIED IN AS 3600. STEEL MESH SHOULD BE LAPPED AS SHOWN BELOW IE THE TWO OUTERMOST CROSS-WIRES OF ONE SHEET SHOUD OVERLAP THE TWO OUTERMOST WIRES AT THE OTHER.



CONTRIBUTE TO LAP LENGTH

- 12. MAINTAIN 30 MIN. COVER TO REINFORCEMENT AT SERVICES AND CAST IN ITEMS THROUGH SLABS AND FOOTINGS. ALL SERVICES THROUGH FOOTING TO BE WRAPPED WITH COMPRESSIBLE MATERIAL MINIMUM 10mm ABLEFLEX OR SIMILAR.
- 13. WELDING OF REINFORCEMENT WILL ONLY BE PERMITTED WITH THE PRIOR APPROVAL OF THE ENGINEER.
- 14. REINFORCEMENT MUST NOT BE CONTINUOUS THROUGH CONTRACTION JOINTS.
- 15. PLACE SUFFICIENT SUPPORT HURDLES/CHAIRS UNDER MAIN BOTTOM REINFORCING BARS AND TOP CROSS BARS IN SLABS AND FOOTINGS TO ALLOW THEM TO BE SUPPORTED IN THEIR CORRECT POSITIONS AND ALIGNMENT DURING CONCRETING (NOT GREATER THAN 900mm CENTRES UNLESS SHOWN OTHERWISE).

16. REINFORCEMENT SYMBOLS:

SYMBOL SPECIFICATION GRADE D500N DEFORMED BAR STRUCTURAL GRADE ROUND BAR

RF AND SL HARD DRAWN SHEET RIBBED WIRE REINFORCING FABRIC THE NUMBER FOLLOWING THESE SYMBOLS IS THE BAR DIAMETER IN MILLIMETERS.

- SLABS TO BE CURED BY COVERING WITH 0.20mm BLACK POLYETHYLENE SHEETING AND KEPT MOIST FOR 7 DAYS MINIMUM OR APPROVED ALTERNATIVE CURING METHOD.
- 18. NO CONCRETE TO BE POURED WHEN SITE TEMPERATURE EXCEEDS 35°C OR FALLS BELOW 5°C.

STRUCTURAL GROUT NOTES

- GROUT IS TO BE HIGH STRENGTH AND NON-SHRINK.
- ALL GROUT TO BE CEMENTITIOUS UNLESS NOTED OTHERWISE.
- THE FOLLOWING GROUT PRODUCT IS TO BE USED UNLESS NOTED OTHERWISE: **CEMENTITIOUS FLOWABLE** GROUT
- 4. WHERE ALTERNATIVE GROUT PRODUCTS ARE USED THE FOLLOWING PROPERTIES ARE TO BE ACHIEVED: COMPRESSIVE STRENGTH (AS1478.2:2005) 55MPa AT 1 DAYS

80MPa AT 7 DAYS FLEXURAL STRENGTH (MODULUS OF RUPTURE AS1012.2.11:2000) 11.4MPa AT 7 DAYS INDIRECT TENSILE STRENGTH (AS1012.2.10:2000) 5.1MPa AT 7 DAYS

- TOP OF CONCRETE TO HAVE A CONCRETE SURFACE PROFILE (CSP) OF MIN 2.0 (GRIND AND WATER BLAST SURFACE) IN ACCORDANCE WITH ICRI TECHNICAL GUIDELINE NO. 310.2R2013.
- ALL GROUT PRODUCTS ARE TO BE STORED, HANDLED AND PLACED STRICTLY IN ACCORDANCE WITH MANUFACTURERS CURRENT TECHNICAL DATA SHEET AND INSTRUCTIONS.

Consulting Engineer:

consulting pty ltd

PROJECT TEAM



GILMOUR SPACE

CONSTRUCTION NOTES:

- REFER GENERAL CONSTRUCTION NOTES ON DWG S01 REFER DRAWING ABBREVIATIONS ON S01
- REFER EARTHWORKS NOTES ON DWG S01

FOR COMMENT 10.03.2023 J.A.W. REV DESCRIPTION DATE BY Status

FOR COMMENT NOT TO BE USED FOR CONSTRUCTION

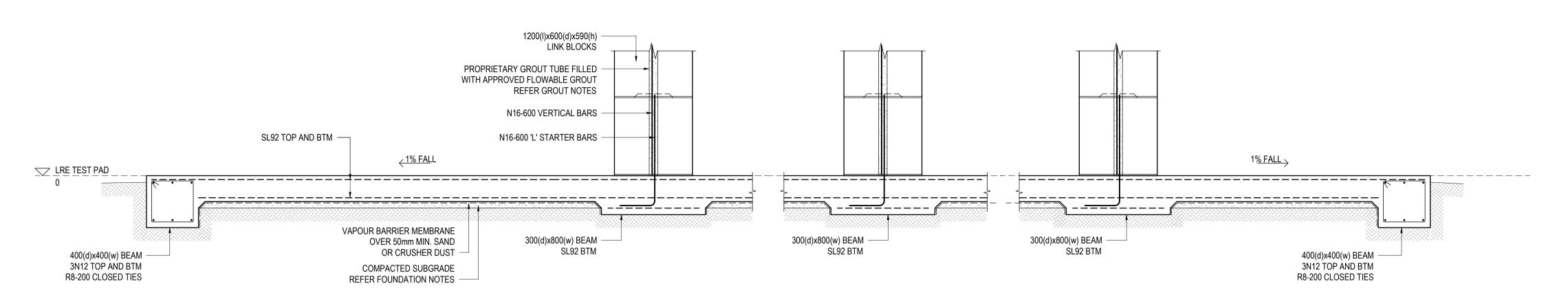
BOWEN ORBITAL SPACE PORT LRE TEST PAD **BOWEN QUEENSLAND**

GILMOUR SPACE TECH

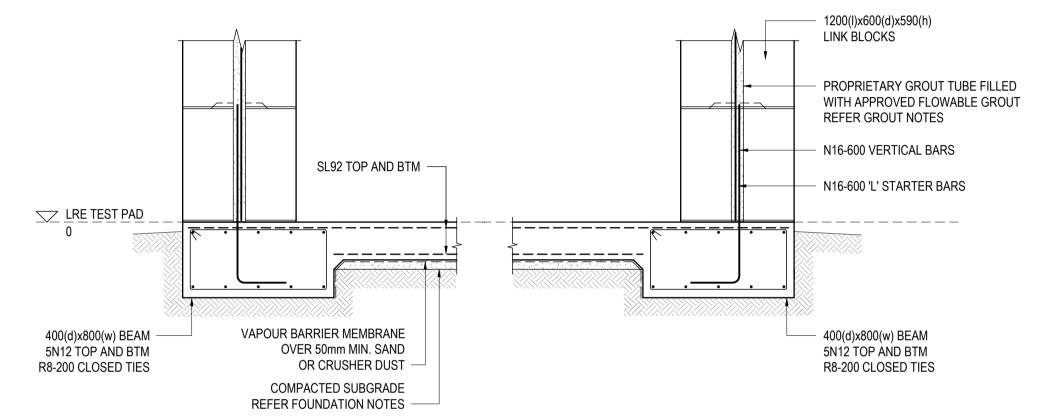
LRE TEST PAD SLAB PLAN

As indicated Project No.	1	Dwg. No. S140	Rev
Scale	A1	Certif	Dat
LV		RM	
Design	Date	Apprd	Dat
JAW		RM	
Drawn	Date	Chkd	Dat

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A SECTION A
S140 1:20



B SECTION B
S140 1:20

PLANS AND DOCUMENTS
referred to in the
SDA APPROVAL

SDA approval: APC2024/006

Consulting Engineer:

consulting

i³ consulting pty ltd engineering consultants innovation, ingenuity, inspiration

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GILMOUR SPACE

A FOR COMMENT 10.03.2023 J.A.W.
REV DESCRIPTION DATE BY

FOR COMMENT
NOT TO BE USED FOR CONSTRUCTION

BOWEN ORBITAL SPACE PORT LRE TEST PAD BOWEN QUEENSLAND

for GILMOUR SPACE TECH

LRE TEST PAD CONCRETE DETAILS

 Drawn
 Date
 Chkd
 Date

 JAW
 RM
 Date
 Date

 LV
 Apprd
 Date

 RM
 Scale
 A1
 Certif
 Date

 1:20
 Dwg. No.
 Rev

 Project No.
 S141
 A

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